



Traffic Impact Assessment

Planning Proposal

Proposed Mixed Use Developments

Parkes and Harris Street, Harris Park

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1. Introduction

TRAFFIX has been commissioned to undertake a Traffic Impact Assessment (TIA) in support of a Planning Proposal relating to three mixed use developments at the following addresses and their respective client:

- Site 1: 114 – 118 Harris Street, Harris Park – Harris Street Development Pty Ltd
- Site 2: 26 – 30 Parkes Street, Harris Park – Parkes St, NSW Pty Ltd
- Site 3: 24 Parkes Street, Harris Park – SH Parkes International Pty Ltd

The development is located within the Parramatta Council LGA and has been assessed under that Council's controls. This report documents the findings of our investigations and should be read in the context of the Statement of Environmental Effects (SEE) prepared separately.

The report is structured as follows:

- Section 2: Describes the site and its location
- Section 3: Documents existing traffic conditions
- Section 4: Describes the concept development
- Section 5: Assesses the parking requirements
- Section 6: Discusses the traffic impacts of the development
- Section 7: Discusses vehicular access requirements
- Section 8: Presents the overall study conclusions.



2. Location and Site

2.1 Location

The sites are situated on the corner of Parkes and Harris Street, Harris Park and lie within the sector bounded by Harris Street to the east, Parkes Street to the south, a mixed use development to the west and Clay Cliff Creek to the north. The subject sites are approximately 400 metres east of Parramatta railway station and approximately 19 kilometres west of the Sydney CBD.

A Location Plan is presented in **Figure 1**, with a Site Plan presented in **Figure 2**, which provide an appreciation of the general character of roads and other key attributes in proximity to the site. Reference should also be made to the photographic record in **Appendix A**.

2.2 Site 1

Site 1 is located at 114-118 Harris Street, Harris Park. It has an irregular configuration and currently accommodates a mixed use development at 114 Harris Street and a medium density residential development at 116 – 118 Harris Street. Site 1 has an eastern site frontage of approximately 40 metres to Harris Street, a southern site boundary of approximately 40 metres to Site 2, a western boundary of approximately 45 metres to Site 3 and a northern boundary of approximately 45 metres to Clay Cliff Creek.

2.3 Site 2

Site 2 is located at 26-30 Parkes Street, Harris Park. It has a rectangular configuration and is currently vacant. Site 2 has an eastern site frontage of approximately 35 metres to Harris Street, a southern site frontage of approximately 40 metres to Parkes Street, a western boundary of approximately 35 metres to Site 3 and a northern boundary of approximately 40 metres to Site 1.

2.4 Site 3

Site 3 is located at 24 Parkes Street, Harris Park. It has a rectangular configuration and is currently vacant. It has an eastern site boundary of approximately 80 metres to Sites 1 and 2, a southern site frontage of approximately 20 metres to Parkes Street, a western boundary of approximately 75 metres



to a neighbouring mixed use development and a northern boundary of approximately 20 metres to Clay Cliff Creek.

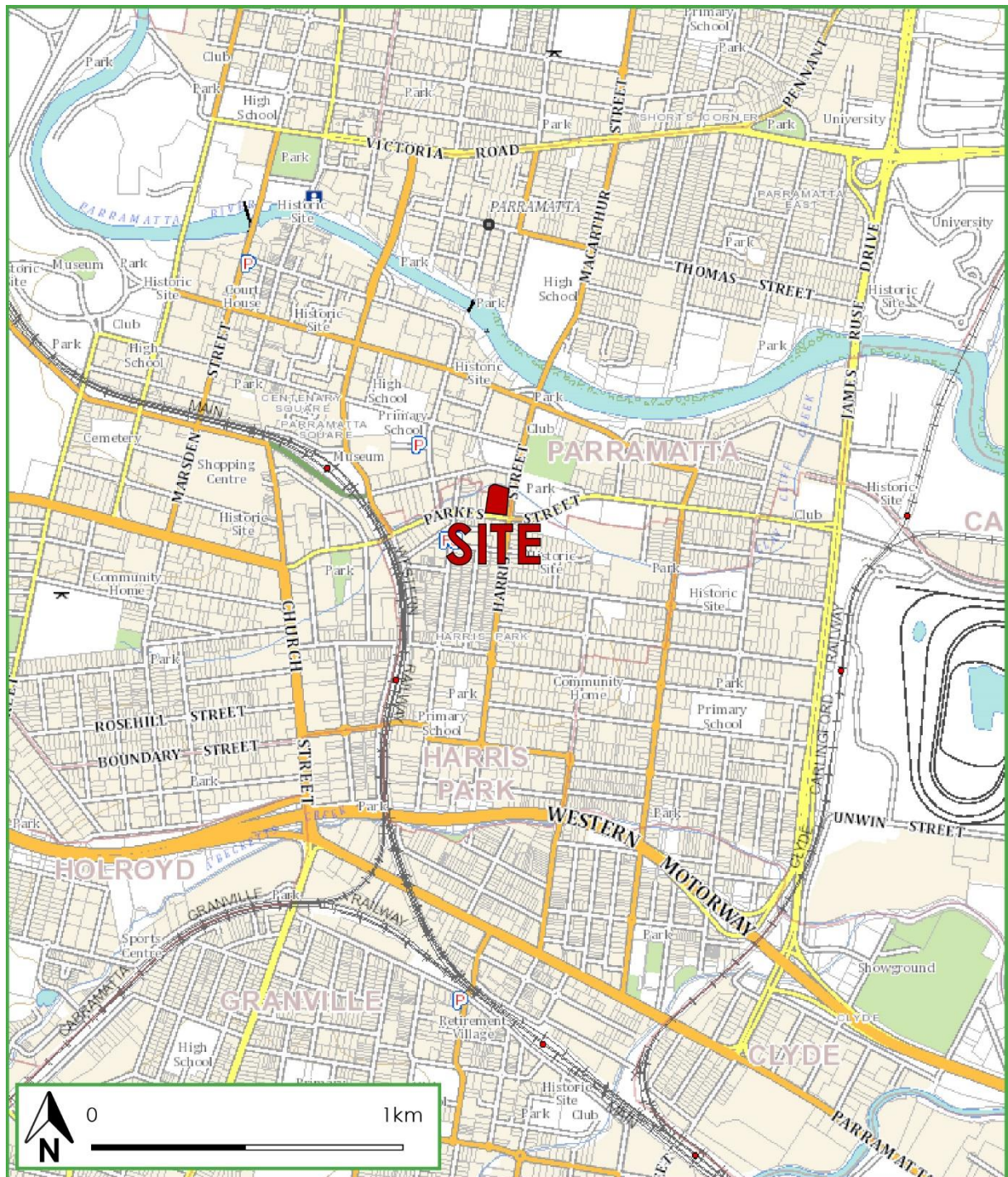


Figure 1: Location Plan



Figure 2: Site Plan



3. Existing Traffic Conditions

3.1 Road Network

The road hierarchy in the vicinity of the site is shown in **Figure 3** with the following roads of particular interest:

- ➊ Parkes Street: an RMS Secondary Road (SR 2049) that runs in an east-west direction between Hassall Street in the east and Parkes Street in the west. Parkes Street is subject to a 60km/h speed zoning and carries two lanes of traffic in each direction within an undivided carriageway. No parking is available along Parkes Street in the vicinity of the site.

- ➋ Harris Street: an Unclassified Regional Road (RR 7484) and local road that runs in a north-south direction between MacArthur Street in the north and forms a cul-de-sac in the south. It is an unclassified regional road between MacArthur Street in the north and Parkes Street, in the south. Harris Street is a local road south of Parkes Street. It permits paid time restricted parallel parking the along the western kerbside in front of the site outside of peak periods. It is subject to a 60km/h speed zoning and carries two lanes of traffic in both directions north of Parkes Street and a 50km/h speed zoning with one lane of traffic in each direction south of Parkes Street.

- ➌ Wigram Street: a local road that runs in a north-south direction between Hassall Street in the north and forms a cul-de-sac in the south. It permits time restricted parallel parking on both kerbsides and is subject to a 50km/h speed zoning. Wigram Street carries a single lane of traffic in both directions.

It can be seen from **Figure 3** that the site is conveniently located with respect to the arterial and local road systems serving the region. It is therefore able to effectively distribute traffic onto the wider road network, minimising traffic impacts.

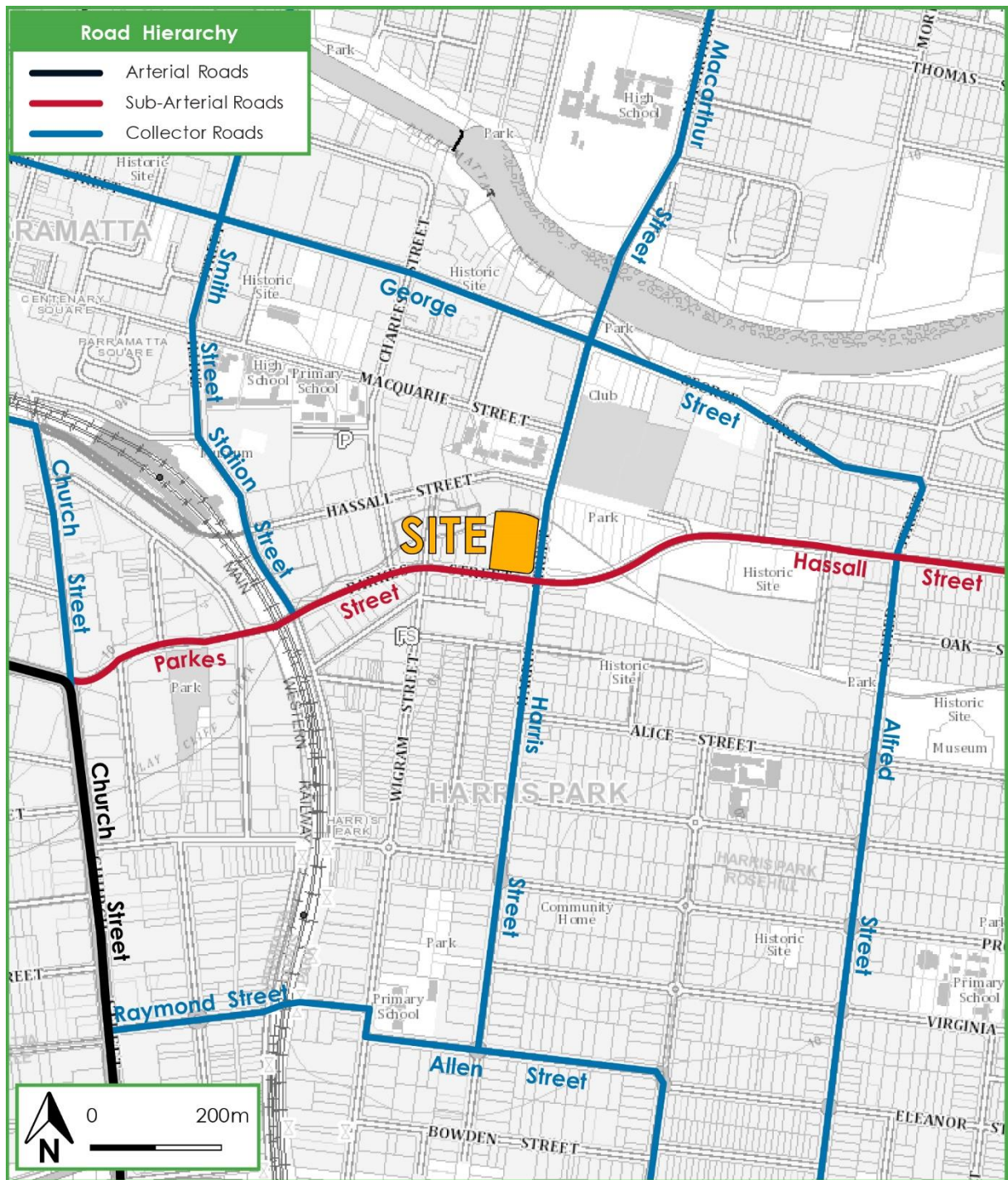


Figure 3: Road Hierarchy



3.2 Key Intersections

The key intersections in the vicinity of the site are shown below and provide an understanding of the existing road geometry and alignment



Figure 4: Intersection of Parkes Street with Harris Street

It can be seen from **Figure 4** that the intersection of Parkes Street and Harris Street form a 4-way signal controlled intersection to the south-east of the site. Figure 4 also shows that a signalised controlled pedestrian crossing is provided across all approaches. No restrictions are applied to all movements with the exception of Parkes Street right turn on the western approach which states “No Right Turn” buses excepted. All approaches and exit lane provide two lanes at the intersection.

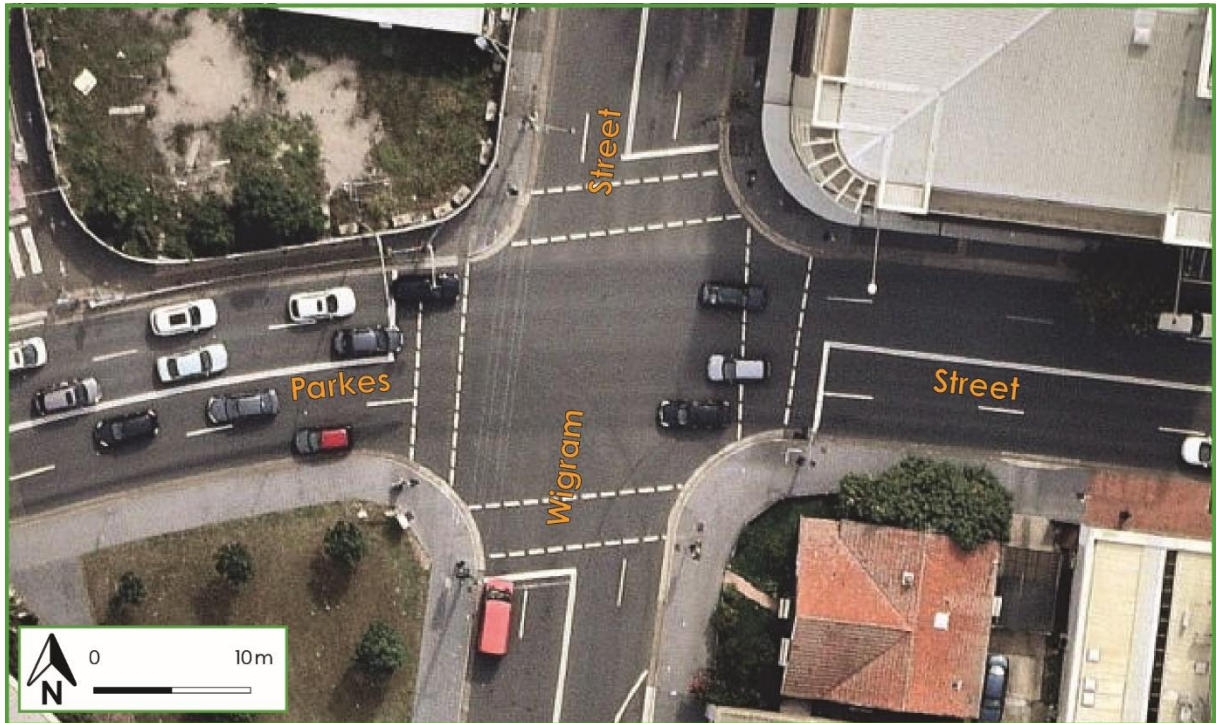


Figure 5: Intersection of Parkes Street with Wigram Street

It can be seen from **Figure 5** that Parkes Street and Wigram Street form a 4-way signal intersection to the south-west of the site. Figure 5 also shows that pedestrian signals are provided across all approaches. No restrictions are applied to all movements with the exception of Parkes Street right turn on the eastern approach which states “No Right Turn” buses excepted. All approaches and exit lane provide two lanes at the intersection.

3.3 Public Transport

The site is well located to take advantage of the numerous public transport services that serve the local area. The existing train and bus services that operate in the locality are shown in **Figure 6**. The site is approximately 400 metres east of Parramatta Railway Station and 600 metres north east of Harris Park Railway Station, which provide services along the T1 Western Line, T2 Inner West Line and T5 Cumberland Line. In addition, there are bus stops within 400 metres walk of the site, providing access to the numerous bus routes that operates in the vicinity of the site providing connections to Macquarie Park, Sutherland, Bankstown and Fairfield.

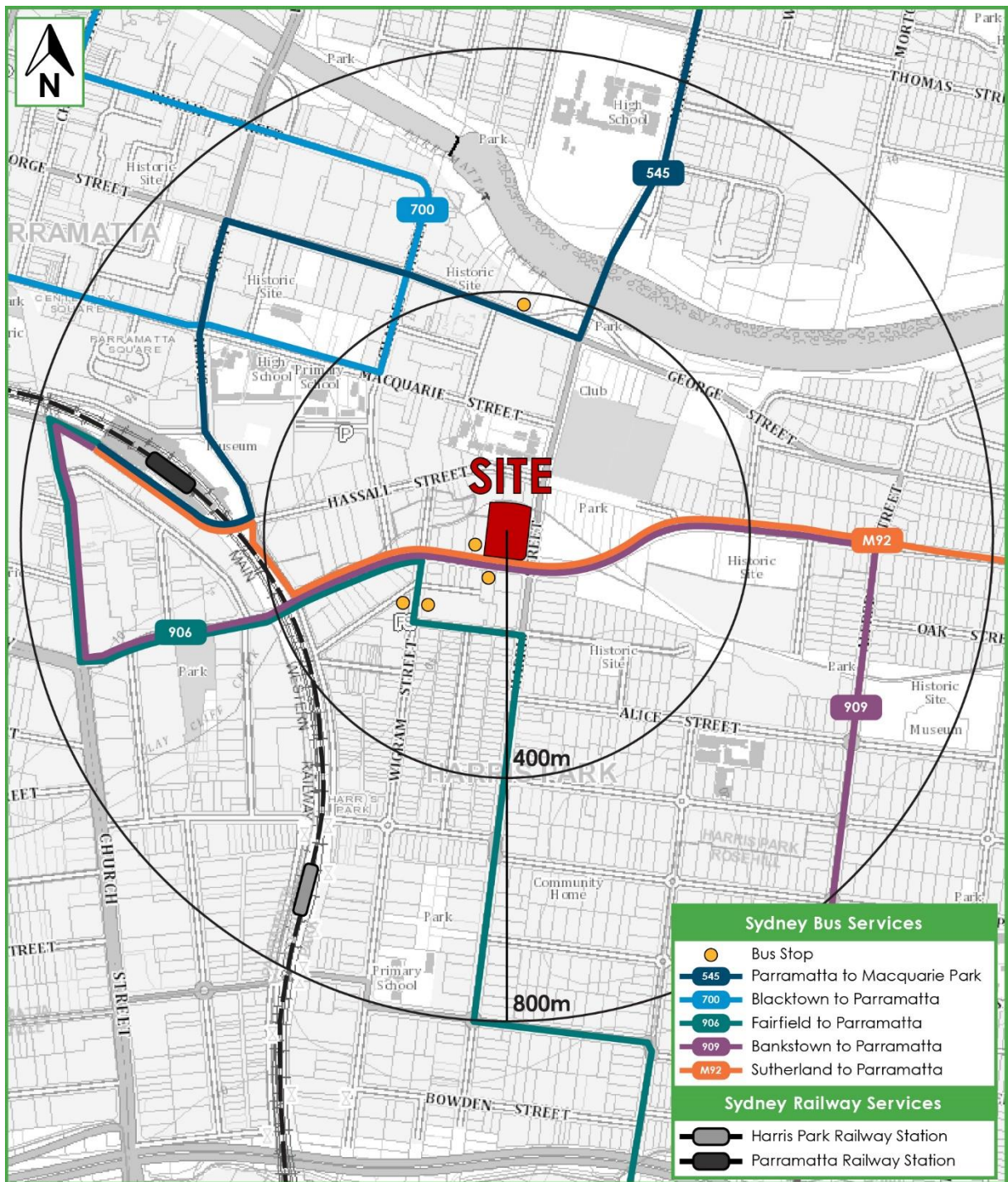


Figure 6: Public Transport



3.4 Existing Site Generation

Based on the RMS Guide to Traffic Generating Developments, a medium density residential dwelling can be expected to generate 0.5 trips / dwelling during the AM and PM peak hours. Application of these rates to the 24 dwellings results in an expected traffic generation of:

- ② 12 vehicle trips during the AM peak hour (2 in, 10 out) and
- ② 12 vehicle trips during the PM peak hour (10 in, 2 out).



4. Concept Development

A detailed description of the changes sought to the LEP can be found in the Planning Proposal, prepared separately. It is understood that the following concept developments would represent the maximum potential of each site:

- ② Site 1 at 114-118 Harris Street proposes 262 apartments, 1,280m² of gross floor area (GFA) for retail use and 1,560m² of GFA for commercial use in a 37-storey building and basement car parking with access from Harris Street. The proposed yield for the apartments are as follows:
 - 35 x one bedroom apartments,
 - 198 x two bedroom apartments,
 - 23 x three bedroom apartments; and
 - 6 x four bedroom apartments.

- ② Site 2 at 26 – 30 Parkes Street will provide 231 apartments, 536m² of GFA for retail use, 1,277m² of GFA for commercial use and 331m² of GFA for a function centre in a 36 storey building and basement car parking with access from Harris Street. The proposed yield for the apartments are as follows:
 - 2 x studio apartments,
 - 70 x one bedroom apartments,
 - 137 x two bedroom apartments; and
 - 22 x three bedroom apartments.

- ② Site 3 at 24 Parkes Street will provide 196 apartments and 1,630m² of GFA for commercial use in a 38 storey building with basement and above ground car parking with access from Parkes Street. The proposed yield for the apartments are as follows:
 - 193 x two bedroom apartments; and
 - 3 x three bedroom apartments.

The parking and traffic impacts arising from the development are discussed in Sections 5 and 6, respectively.



5. Parking Requirements

5.1 Council Controls

The Parramatta Central Business District Strategic Transport Study dated 10 April 2017 resolved that “Council endorses the action recommended by the Parramatta CBD Strategic Transport Study to reduce maximum car parking rates to levels currently used by City of Sydney CBD.” Accordingly, the parking requirements of the concept developments have been assessed against the parking rates of the City of Sydney Local Environmental Plan 2012, using the Category A rates for residential parking, and are summarised in **Table 1** below. The maximum provision for each site is discussed in the following subsections.

Table 1: Council Maximum Parking Rates

| Type | Council DCP Parking Rates |
|-------------|--|
| Retail | See Formula * |
| Commercial | |
| Residential | 0.1 spaces per studio dwelling |
| | 0.3 spaces per 1-bedroom dwelling |
| | 0.7 spaces per 2-bedroom dwelling |
| | 1 space per 3 or more bedroom dwelling |

* Maximum Retail and Office parking requirements under CoS LEP

The required parking is to be calculated using the following formula: $M = (G \times A) / (50 \times T)$ where:

M is the maximum number of parking spaces, and

G is the gross floor area of all retail premises in the building in square metres, and

A is the site area in square metres, and

T is the total gross floor area of all buildings on the site in square metres.

5.1.1 Site 1: Maximum Parking Provision

The concept development for Site 1 will have a maximum parking provision according to **Table 2** in accordance with the Parramatta CBD Strategic Transport Study.



Table 2: Maximum Parking Provision for Site 1

| Type | Area (GFA) / No | Council DCP Parking Rates | Maximum allowable spaces |
|-------------|----------------------|--|--------------------------|
| Retail | 1,280m ² | See Formula * A = 1,776m ² T = 25,675m ² | 1.8 |
| Commercial | 1,560 m ² | | 2.2 |
| Residential | - | 0.1 spaces per studio dwelling | - |
| | 35 | 0.3 spaces per 1-bedroom dwelling | 11 |
| | 198 | 0.7 spaces per 2-bedroom dwelling | 139 |
| | 29 | 1 space per 3 or more bedroom dwelling | 29 |
| Totals | | | 183 |

It can be seen from Table 2 that the development is permitted to provide a maximum of 183 spaces under Council's controls for the retail, commercial and residential components of the concept development.

5.1.2 Site 2: Maximum Parking Provision

The concept development for Site 2 will have a maximum parking provision according to **Table 3** in accordance with the Parramatta CBD Strategic Transport Study.

Table 3: Maximum parking Provision for Site 1

| Type | Area (GFA) / No | Council DCP Parking Rates | Maximum allowable spaces |
|---------------------|--------------------|--|--------------------------|
| Retail | 363m ² | See Formula * A = 1,493m ² T = 21,375m ² | 3 |
| Function | 386m ² | | |
| Commercial | 1265m ² | | |
| Serviced Apartments | 12 | 1 space for every 4 bedrooms up to 100 bedrooms | 3 |
| Residential | 2 | 0.1 spaces per studio dwelling | 0.2 |
| | 70 | 0.3 spaces per 1-bedroom dwelling | 21 |
| | 137 | 0.7 spaces per 2-bedroom dwelling | 96 |
| | 22 | 1 space per 3 or more bedroom dwelling | 22 |
| Totals | | | 146 |



It can be seen from Table 3 that the development is permitted to provide a maximum of 146 spaces under Council's controls for the residential component of the concept development.

5.1.3 Site 3: Maximum Parking Provision

The concept development for Site 3 will have a maximum parking provision according to **Table 4** in accordance with the Parramatta CBD Strategic Transport Study.

Table 4: Maximum parking Provision for Site 1

| Type | Area (GFA) / No | Council DCP Parking Rates | Maximum allowable spaces |
|-------------|----------------------|--|--------------------------|
| Commercial | 1,630 m ² | See Formula * A = 1,631m ² T = 18,756m ² | 2.8 |
| Residential | - | 0.1 spaces per studio dwelling | - |
| | - | 0.3 spaces per 1-bedroom dwelling | - |
| | 193 | 0.7 spaces per 2-bedroom dwelling | 135 |
| | 3 | 1 space per 3 or more bedroom dwelling | 3 |
| Totals | | | 141 |

It can be seen from Table 4 that the development is permitted to provide a maximum of 141 spaces under Council's controls for the commercial and residential components of the concept development.

5.2 Accessible Parking

Schedule 7.8.5 of the City of Sydney Council's DCP 2012 states the following requirements with regard to accessible parking:

- ➊ One (1) accessible car parking space is to be provided for every adaptable residential unit.
- ➋ One (1) space for every 20 car parking spaces or part thereof is to be allocated as accessible visitor parking.
- ➌ The space shall meet the requirements of AS2890.6 providing an adjacent 'shared zone' of 2.4m x 5.4m to assist with loading and unloading.



- For residential development, accessible car parking spaces are to be allocated to adaptable units, or as visitor parking. Accessible car parking spaces allocated to adaptable dwelling units are to be a part lot to an adaptable unit in the strata plan.

With regards to the subject development, the site location within land use Category A precludes the provision of visitor parking on site. As such, accessible visitor spaces are not required.

5.3 Bicycle Parking

Part 4 of Council's City Centre DCP requires provision for secure bicycle parking at a rate of one (1) bicycle parking space per 200m² of commercial / retail GFA or part thereof and one (1) bicycle parking space for every two (2) dwellings. The bicycle parking spaces are to be provided in accordance with security level B under AS2890.3 which requires a secure room or structure to contain the bicycle parking spaces. The provision of end of trip facilities including lockers and showers for retail and commercial uses must be provided.

5.4 Motorcycle Facilities

Council's DCP requires an area equal to a minimum of one motorcycle space to be provided as separate parking for motorcycles for every 25 on-site car parking spaces provided, or part thereof.

5.5 Servicing

The RMS *Guide to Traffic Generating Developments* recommends the following service vehicle parking bays be provided at the following rates:

- Commercial (50% for trucks)
 - 1 spaces per 4,000m² for the first 20,000m² GFA, plus
 - 1 space per 8,000m² over 20,000m² GFA
- Retail (all spaces for trucks)
 - 5 + 1 space per 1,000m² for more than 2,000m² GFA



As such, no development will require a loading dock however smaller servicing for up to a B99 vehicle or 6.4m long small rigid vehicle could be accommodated on site for occasional servicing such as private waste collection and deliveries. However this will be depend on the requirements of each development and further analysis can be provided at development application stage.

If on site servicing is required, a Loading Dock Management Plan can be prepared by building management to ensure that demands for service vehicles bays is appropriately managed and this can be conditioned as part of a consent for a future development application. It is expected that this Management Plan would restrict service vehicle access to the site outside of peak periods to reduce potential conflicts with cars using the basement car park.



6. Traffic Impacts

6.1 Existing Intersection Performance

For the purposes of assessment of the traffic impacts of the concept developments, surveys were undertaken on a typical weekday in 2018 of the most critical intersections adjacent to the site during the network peaks between 7:00am and 9:00am and 4:00pm and 6:00pm, being:

- Parkes Street / Harris Street,
- Parkes Street / Wigram Street,

It is noted that the results of the surveys indicated that the network peak hour occurred at 7:45am – 8:45am during the morning (AM) and 4:30pm – 5:30pm during the evening (PM). The results of these surveys were analysed using the SIDRA Intersection 8 computer program to determine their performance characteristics under existing traffic conditions. The SIDRA model produces a range of outputs, the most useful of which are the Degree of Saturation (DOS) and Average Vehicle Delay per vehicle (AVD). The AVD is in turn related to a level of service (LOS) criteria. These performance measures can be interpreted using the following explanations:

DOS - the DOS is a measure of the operational performance of individual intersections. As both queue length and delay increase rapidly as DOS approaches 1, it is usual to attempt to keep DOS to less than 0.9. When DOS exceeds 0.9 residual queues can be anticipated, as occurs at many major intersections throughout the metropolitan area during peak periods. For intersections controlled by roundabout or give way/stop control, satisfactory intersection operation is generally indicated by a DOS of 0.8 or less.

AVD - the AVD for individual intersections provides a measure of the operational performance of an intersection. In general, levels of acceptability of AVD for individual intersections depend on the time of day (motorists generally accept higher delays during peak commuter periods) and the road system being modelled (motorists are more likely to accept longer delays on side streets than on the main road system).

LOS - this is a comparative measure which provides an indication of the operating performance of an intersection as shown below:



| Level of Service | Average Delay per Vehicle (secs/veh) | Traffic Signals, Roundabout | Give Way and Stop Signs |
|------------------|--------------------------------------|---|--|
| A | less than 14 | Good operation | Good operation |
| B | 15 to 28 | Good with acceptable delays and spare capacity | Acceptable delays and spare capacity |
| C | 29 to 42 | Satisfactory | Satisfactory but accident study required |
| D | 43 to 56 | Operating near capacity | Near capacity and accident study required |
| E | 57 to 70 | At capacity; at signals incidents will cause excessive delays. Roundabouts require other control mode | At capacity and requires other control mode |
| F | More than 70 | Unsatisfactory and requires additional capacity. | Unsatisfactory and requires other control mode or major treatment. |

A summary of the modelled results are provided below in **Table 5**. Reference should also be made to the SIDRA outputs provided in **Appendix C** which provide detailed results for individual movements and approaches.

Table 5: Existing Intersection Performances

| Intersection | Control Type | Period | Degree of Saturation | Intersection Delay | Level of Service |
|-------------------------------|--------------|--------|----------------------|--------------------|------------------|
| Parkes Street / Harris Street | Signals | AM | 1.004 | 55.4 | D |
| | | PM | 1.056 | 71.6 | F |
| Parkes Street / Wigram Street | Signals | AM | 0.653 | 27.7 | B |
| | | PM | 0.987 | 55.1 | D |

It can be seen from **Table 5** that the intersection of Parkes Street with Harris Street do not operate satisfactorily under the existing 'base case' scenario during the PM peak, with a Levels of Service (LOS) F. However, during the AM peak, Parkes Street with Harris operates with a LOS D being near capacity. The intersection of Parkes Street with Wigram Street operates with a LOS B and D during the AM and PM peak periods, respectively. Nevertheless, it is stressed that the most relevant use of this analysis is to compare the relative change in the performance parameters as a result of the concept development. This is discussed further in **Section 6**.



6.2 Trip Generation

The traffic generation rates used to determine the traffic generation are discussed below. The rates will then be used to determine the traffic generation for each component of each of the concept developments.

6.2.1 Residential

The RMS Technical Direction (TDT 2013/04a) provides traffic generation rates for high density residential use based upon surveys conducted during 2012. It recommends an average Sydney trip rate of 0.19 vehicle trips per unit during the AM peak hour and 0.15 vehicle trips per unit during the PM peak hour. The traffic is assumed to have a 20:80 split for in and out during the AM peak and vice versa during the PM peak.

Application of the above rates to the proposed 262 residential units for Site 1 results in an expected traffic generation of:

- ➊ 50 vehicle trips per hour during the AM peak period (10 in, 40 out); and
- ➋ 39 vehicle trips per hour during the PM peak period (31 in, 8 out).

Application of the above rates to the proposed 231 residential units for Site 2 results in an expected traffic generation of:

- ➊ 44 vehicle trips per hour during the AM peak period (9 in, 35 out); and
- ➋ 35 vehicle trips per hour during the PM peak period (28 in, 7 out).

Application of the above rates to the proposed 196 residential units for Site 3 results in an expected traffic generation of:

- ➊ 38 vehicle trips per hour during the AM peak period (8 in, 30 out); and
- ➋ 30 vehicle trips per hour during the PM peak period (24 in, 6 out).



6.2.2 Retail

The RMS Guide to Traffic Generating Developments specifies a traffic generation rate for retail speciality shops of 4.6 vehicle trips per hour per 100m² of GFA. The rate for the AM Peak was discounted to 25% of the full rate as customers are unlikely to be shopping during this period and the discounted would only account for staff and deliveries. The rate for the PM peak was discounted by 50% as the proposed retail will be catering mostly to local residents and employees who would walk to the retail shops. Therefore, the discounted rates for the retail component of the developments are considered a supportable estimate. The traffic is assumed to have a 50:50 split for in and out during the AM and PM peaks.

Application of the above rates to the proposed 1,280m² of GFA for Site 1 results in an expected traffic generation of:

- ➊ 15 vehicle trips per hour during the AM peak period (8 in, 7 out); and
- ➋ 29 vehicle trips per hour during the PM peak period (15 in, 14 out).

Application of the above rates to the proposed 536m² of GFA for Site 2 results in an expected traffic generation of:

- ➊ 4 vehicle trips per hour during the AM peak period (2 in, 2 out); and
- ➋ 8 vehicle trips per hour during the PM peak period (4 in, 4 out).

6.2.3 Commercial

The RMS Technical Direction (TDT 2013/04a) specifies a traffic generation rate for office blocks of 1.6 vehicle trips per 100m² during the AM peak hour and 1.2 vehicle trips per 100m² during the PM peak hour. However Appendix D of the Technical Direction specifies the rate for the different suburbs around Sydney and there is significant variation between the traffic generation rates at different locations. As such, the rate for Parramatta has been assumed for the sites which is considered a more accurate assessment. The rates assumed are 0.69 vehicles per 100m² during the AM peak hour and 0.28 vehicles per 100m² during the PM peak hour. The traffic is assumed to have an 80:20 split for in and out during the AM peak and vice versa during the PM peak.

Application of the above rates to the proposed 1,560m² of GFA for Site 1 results in an expected traffic generation of:



- ② 11 vehicle trips per hour during the AM peak period (9 in, 2 out); and
- ② 4 vehicle trips per hour during the PM peak period (1 in, 3 out).

Application of the above rates to the proposed 1,277m² of GFA for Site 2 results in an expected traffic generation of:

- ② 9 vehicle trips per hour during the AM peak period (7 in, 2 out); and
- ② 4 vehicle trips per hour during the PM peak period (1 in, 3 out).

Application of the above rates to the proposed 1,630m² of GFA for Site 3 results in an expected traffic generation of:

- ② 11 vehicle trips per hour during the AM peak period (9 in, 2 out); and
- ② 5 vehicle trips per hour during the PM peak period (1 in, 4 out).

6.2.4 Function Centre

The function centre is assumed to operate outside of the network peaks of 7:45am – 8:45am and 4:30pm – 5:30pm. Therefore, the traffic generation for the function centre has not been included in the analysis of the intersections as the peak period of the function centre and network peak do not overlap.

6.2.5 Serviced Apartments

The RMS *Guide to Traffic Generating Developments* does not specify trip generation rates for serviced apartments, however recommends a rate of 0.4 vehicle trips per unit for a motel. Application of this rate to the 12 proposed serviced apartments ordinarily results in five (5) vehicle trips per hour being generated. However, due the parking provision allowing a maximum of three (3) parking spaces the traffic generation is assumed to be less than the motel rate and in line with the number of parking spaces provided. On this basis, the estimated traffic generation for this component is as follows:

- ② 3 vehicle trips per hour during the AM peak period (0 in, 3 out); and
- ② 3 vehicle trips per hour during the PM peak period (3 in, 0 out).



6.2.6 Summary of Traffic Generation Rates

Table 6 provides a summary of the traffic generation rates for each of the proposed uses of the sites as described in the previous subsections. **Table 7** provides a summary of the traffic generation for each use of each site using the rates in Table 6.

Table 6: Summary of Traffic Generation Rates

| Land Use | AM Peak | | | PM Peak | | |
|--------------------------|--------------------------|-----|------|--------------------------|------|-----|
| | Traffic Generation Rate | IN | OUT | Traffic Generation Rate | IN | OUT |
| High Density Residential | 0.19 / unit | 20% | 80% | 0.15 / unit | 80% | 20% |
| Commercial | 0.69 / 100m ² | 80% | 20% | 0.28 / 100m ² | 20% | 80% |
| Retail | 1.15 / 100m ² | 50% | 50% | 2.3 / 100m ² | 50% | 50% |
| Serviced Apartments | 2 / 5 units | 0% | 100% | 1 / 6 units | 100% | 0% |

Table 7: Summary of Traffic Generation for Each Site and Use

| Land Use | | No. / Area | AM Peak | | | PM Peak | | |
|---------------------|--------|------------|----------|----|-----|----------|----|-----|
| | | | COMBINED | IN | OUT | COMBINED | IN | OUT |
| Residential | Site 1 | 262 | 50 | 10 | 10 | 39 | 31 | 8 |
| | Site 2 | 231 | 44 | 9 | 35 | 35 | 28 | 7 |
| | Site 3 | 199 | 38 | 8 | 30 | 30 | 24 | 6 |
| Commercial (GFA) | Site 1 | 1,560 | 11 | 9 | 2 | 4 | 1 | 3 |
| | Site 2 | 1,277 | 9 | 7 | 2 | 4 | 1 | 3 |
| | Site 3 | 1,630 | 11 | 9 | 2 | 5 | 1 | 4 |
| Retail (GLA) | Site 1 | 1,280 | 15 | 8 | 7 | 29 | 15 | 14 |
| | Site 2 | 363 | 4 | 2 | 2 | 8 | 4 | 4 |
| Serviced Apartments | Site 2 | 12 | 3 | 0 | 3 | 3 | 3 | 0 |
| Total | Site 1 | | 76 | 26 | 50 | 74 | 48 | 26 |
| | Site 2 | | 60 | 18 | 42 | 50 | 36 | 14 |
| | Site 3 | | 49 | 17 | 32 | 35 | 25 | 10 |



6.2.7 Combined Traffic Generation

Having consideration for the above the total traffic generation for each site is as follows.

Site 1 is expected to have a traffic generation of:

- 76 vehicle trips per hour during the AM peak period (26 in, 50 out); and
- 74 vehicle trips per hour during the PM peak period (48 in, 26 out).

Site 2 is expected to have a traffic generation of:

- 60 vehicle trips per hour during the AM peak period (18 in, 42 out); and
- 50 vehicle trips per hour during the PM peak period (36 in, 14 out).

Site 3 is expected to have a traffic generation of:

- 49 vehicle trips per hour during the AM peak period (17 in, 32 out); and
- 35 vehicle trips per hour during the PM peak period (25 in, 10 out).

It should be noted that existing traffic generation for Site 1 as discussed in Section 3.4 has not been taken into account. Therefore, the following analysis is considered conservative assessment as this will not be deducted from the assessment.

6.3 Trip Distribution

6.3.1 Residential and Commercial Traffic Distribution

The relative distribution of 2011 Journey-to-Work trips by car for the area in the vicinity of the site (Travel Zone 1057) has been used to determine the future distribution of traffic to and from the developments on the surrounding road network for the residential and commercial uses. In this regard, the localised distribution of this traffic onto the surrounding road network is summarised in **Table 8** below.



Table 8: Traffic Distribution for Residential and Commercial Traffic

| Direction | Vehicles Percentage | | Location (To/From) |
|---|----------------------------------|-----------------------------|---|
| | Employed residents travelling to | Employed people coming from | |
| Harris Street (North) | 21% | 21% | Baulkham Hills and Hawkesbury, Parramatta |
| Harris Street (South) – excl. Site 3 exit | 7% | 4% | Parramatta |
| Parkes Street (East) heading south – Site 3 exit only | | | |
| Parkes Street (East) | 35% | 34% | City and Inner South, Eastern Suburbs, Inner South West, Inner West, North Sydney and Hornsby, Northern Beaches, Ryde, Sutherland |
| Parkes Street (West) | 37% | 41% | Blacktown, Outer South West, Outer West and Blue Mountains, Parramatta |

6.3.2 Retail and Serviced Apartments Traffic Distribution

It is assumed that the retail and serviced apartment traffic will arrive and depart using the two arterial roads to and from the site, which are Harris Street to the north and South and Parkes Street to the east and west. **Table 9** shows the assumed distribution for each direction. It was assumed the three arterial directions North, East and West would have equal traffic arrivals and as the south direction is a local road this was assumed to have a lower distribution. This is considered appropriate for the serviced apartments as the low distribution to the south results in no traffic from this direction and therefore all traffic from arterial roads, which is considered a reasonable assumption

Table 9: Traffic Distribution for Retail Traffic

| Direction | Assumed Distribution | |
|-----------------------|----------------------|-----|
| | In | Out |
| Harris Street (North) | 30% | 30% |
| Harris Street (South) | 10% | 10% |
| Parkes Street (East) | 30% | 30% |
| Parkes Street (West) | 30% | 30% |



6.3.3 Site Traffic Distributions

Based on the above traffic distributions **Figure 7** and **Figure 8** below show the traffic generation for each site and the direction of all vehicles entering and exiting the sites during the AM and PM peak hours.

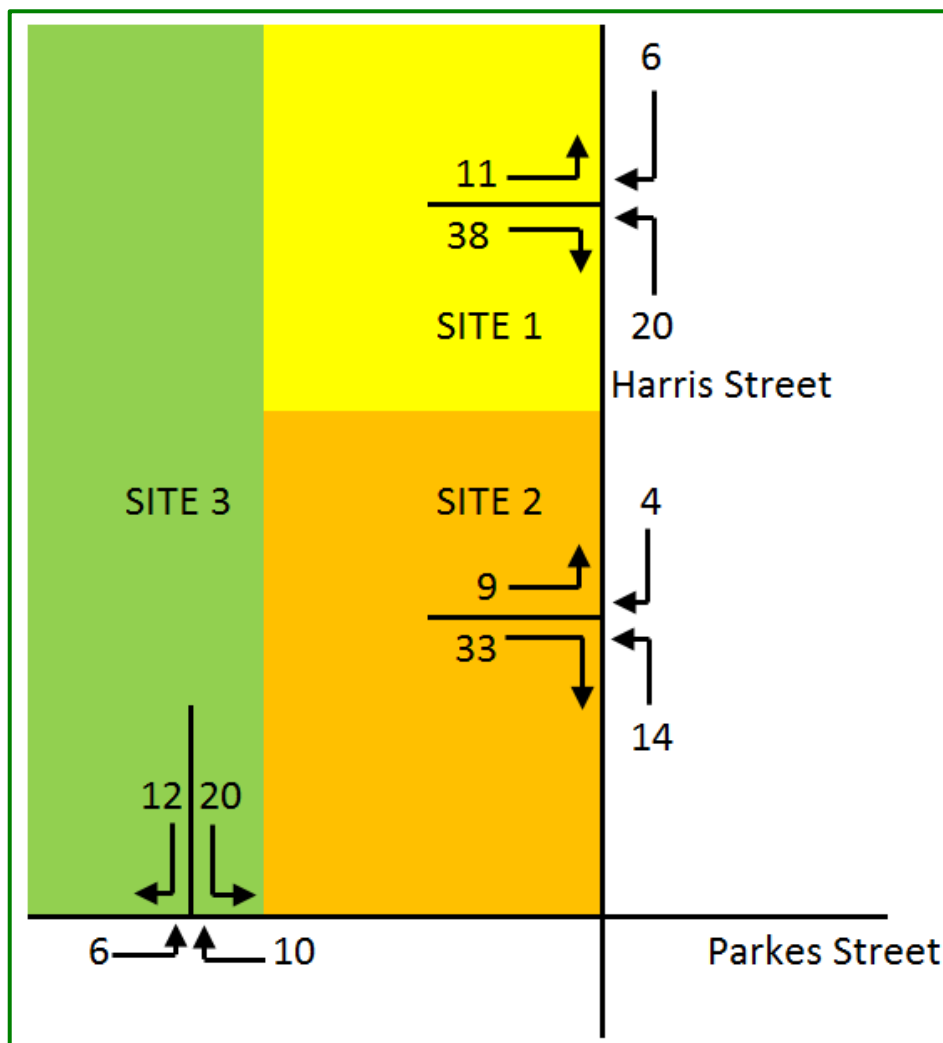


Figure 7: Site Traffic Distributions during the AM Peak Hour

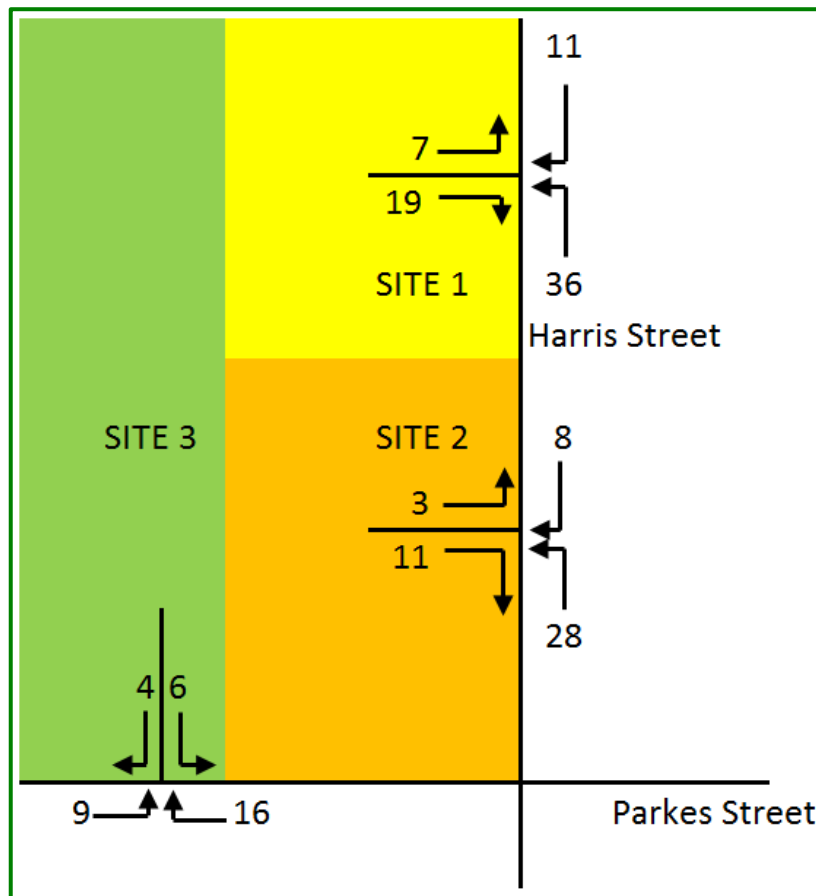


Figure 8: Site Traffic Distributions during the PM Peak Hour

6.3.4 Intersection Traffic Distribution

Based on the distribution on the above Figures, **Figure 9** and **Figure 10** below show the distributions of the traffic generated by the three concept developments at the two key intersections in the vicinity of the sites during the AM and PM peak hours.

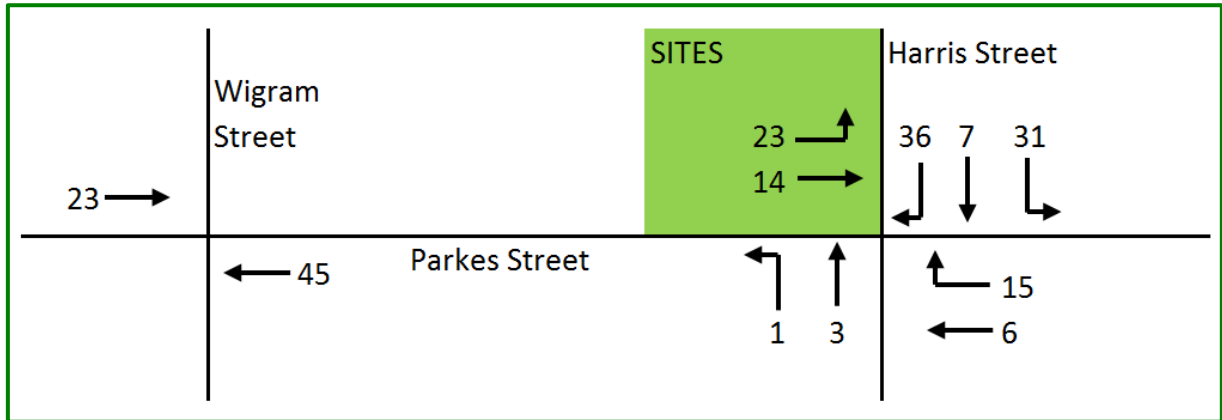


Figure 9: Intersection Traffic Distributions during the AM Peak Hour

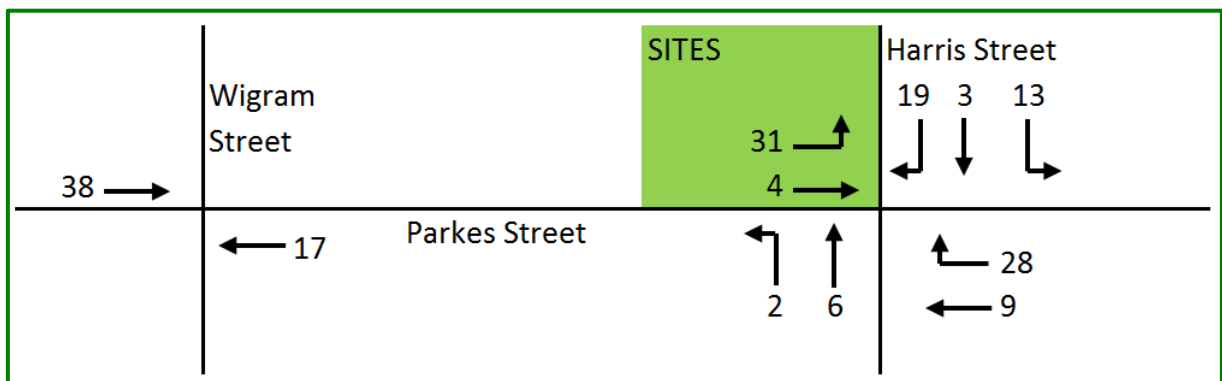


Figure 10: Intersection Traffic Distributions during the AM Peak Hour

6.4 Peak Period Intersection Performances

6.4.1 Existing + Developments Model (No Improvements)

The traffic distribution in Figure 9 and Figure 10 have been applied to the existing network models from Section 6.1 to determine the operation of the future network with the concept developments. A summary of the modelled results are provided in **Table 10** below. Reference should also be made to the SIDRA outputs provided in **Appendix C** which provide detailed results for individual movements and approaches.



Table 10: Intersection Performance - Existing + Developments

| Intersection Description | Control Type | Period | Model | Degree of Saturation | Intersection Delay | Level of Service |
|-------------------------------|--------------|--------|------------------|----------------------|--------------------|------------------|
| Parkes Street / Harris Street | Signals | AM | Existing | 1.004 | 55.4 | D |
| | | AM | With Development | 1.093 | 89.6 | F |
| | | PM | Existing | 1.056 | 71.6 | F |
| | | PM | With Development | 1.111 | 94.8 | F |
| Parkes Street / Wigram Street | Signal | AM | Existing | 0.653 | 27.7 | B |
| | | AM | With Development | 0.781 | 30.1 | C |
| | | PM | Existing | 0.987 | 55.1 | D |
| | | PM | With Development | 1.168 | 124.2 | F |

It can be seen from **Table 6** that the intersections do not operate satisfactorily under the future scenario, with a level of service F during both peak periods for the intersection of Harris Street and Parkes Street and for Parkes Street and Wigram Street in the PM peak hour. Therefore, improvements are proposed to improve the operation of the intersections with the proposed traffic generation.

6.4.2 Existing + Developments with Improvements

To improve the performance of the intersections with the additional development traffic, changes to the cycle times and phase sequence of both traffic signals are proposed. **Figure 11** and **Figure 12** show the proposed phasing for each intersection. The input phase sequence and phasing summary outputs for both intersections during the AM and PM peaks are included in Appendix C. A summary of the modelled results are provided in **Table 11** below. Reference should also be made to the SIDRA outputs provided in **Appendix C** which provide detailed results for individual lanes and approaches.

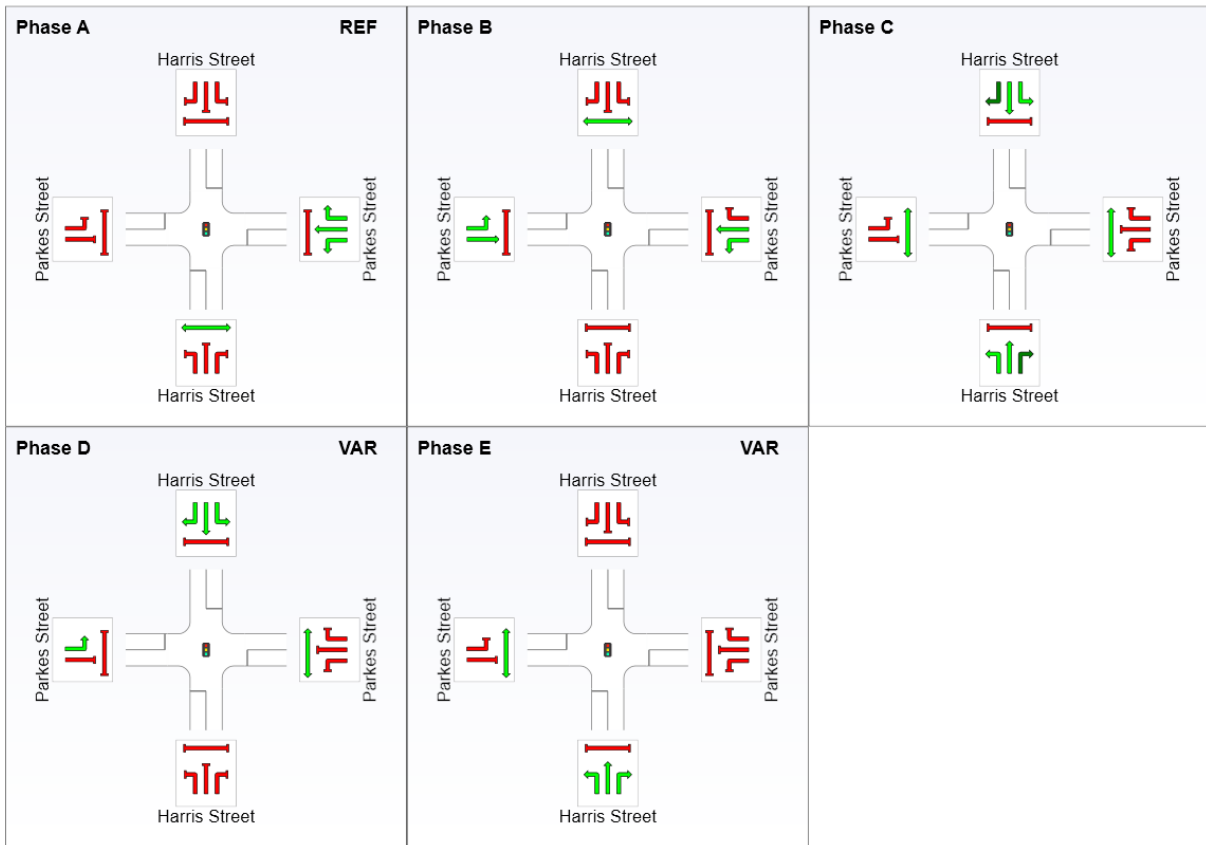


Figure 11: Proposed Phasing Input for the Intersection Harris Street and Parkes Street

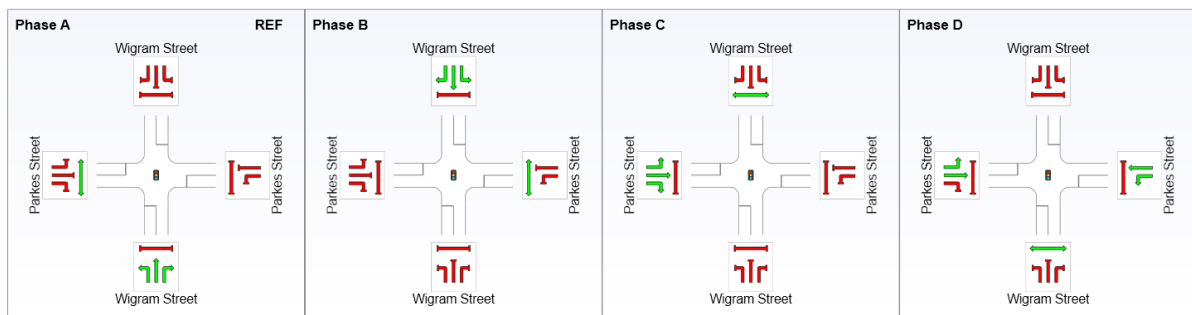


Figure 12: Proposed Phasing Input for the Intersection Wigram Street and Parkes Street



Table 11: Intersection Performance - Existing + Development with Improvements

| Intersection | Control Type | Period | Degree of Saturation | Intersection Delay | Level of Service |
|-------------------------------|--------------|--------|----------------------|--------------------|------------------|
| Parkes Street / Harris Street | Signals | AM | 1.004 | 58.4 | E |
| | | PM | 1.148 | 59.0 | E |
| Parkes Street / Wigram Street | Signals | AM | 0.754 | 28.2 | B |
| | | PM | 0.892 | 35.4 | C |

It can be seen from **Table 11** that the intersections operate significantly better under the future with improvements scenario for the cumulative assessment, with a LOS E during both peak periods for the intersection of Harris Street and Parkes Street. The intersection of Parkes Street and Wigram Street now operates satisfactorily with a LOS C or better during both peak periods. Therefore, the improvements are recommended to be adopted to improve existing traffic flow, particularly during the PM peak and allow for the increased traffic generation with the concept developments. Accordingly, the traffic impacts associated with the developments can be accommodated on the road network with the proposed changes to the timing cycle and phase sequence.



7. Vehicular Access

7.1 Access Requirements

The concept developments are required provide vehicular accesses in accordance with AS 2890.1 (2004). This will depend on the requirements of each development and further analysis can be provided at development application stage. The following requirements are noteworthy for each site.

7.2 Site 1

With a maximum parking provision of 183 'Class 1A' car parking spaces with access on a local road (Harris Street), the development is required to provide a 'Category 2' driveway under AS2890.1. This requires a combined entry exit driveway of 6.0m – 9.0m.

7.3 Site 2

With a maximum parking provision of 146 'Class 1A' car parking spaces with access on a local road (Harris Street), the development is required to provide a 'Category 2' driveway under AS2890.1. This requires a combined entry exit driveway of 6.0m – 9.0m.

7.4 Site 3

With a maximum parking provision of 141 'Class 1A' car parking spaces with access on an arterial road (Parkes Street), the development is required to provide a 'Category 3' driveway under AS2890.1. This requires separate entry and exit driveways of 6.0m and 4.0m – 6.0m, respectively. However, if the development provides 100 parking spaces or less the development can provide a Category 2 driveway which requires a combined entry exit driveway of 6.0m – 9.0m.



8. Conclusions

In summary:

- ② TRAFFIX has been commissioned to undertake a Traffic Impact Assessment (TIA) in support of a Planning Proposal relating to three mixed use developments at the following addresses and their respective client
 - 114 – 118 Harris Street, Harris Park – Harris Street Development Pty Ltd
 - 26 – 30 Parkes Street, Harris Park – Parkes St, NSW Pty Ltd
 - 24 Parkes Street, Harris Park – SH Parkes International Pty Ltd

- ② A detailed description of the concept development is provided in the Statement of Environmental Effects prepared separately. In summary, the developments for which approval is sought comprise the following components:
 - Site 1 at 114-118 Harris Street proposes 262 apartments, 1,150m² of gross floor area (GFA) for retail use and 1,560m² of GFA for commercial use in a 537-storey building and basement car parking with access from Harris Street.
 - Site 2 at 26-30 Parkes Street will provide 231 apartments, 12 serviced apartments, 363m² of GFA for retail use, 1,265m² of GFA for commercial use and 386m² of GFA for a function centre in a 36 storey building and basement car parking with access from Harris Street.
 - Site 3 at 24 Parkes Street will provide 199 apartments and 1,630m² of GFA for commercial use in a 38 storey building with basement and above ground car parking with access from Parkes Street.

- ② The maximum parking requirements for the concept developments have been provided in accordance with the City of Sydney Local Environmental Plan 2012 as required by Parramatta City Council for development within the Parramatta City Centre.

- ② The three developments will generate the following traffic during the AM and PM peak hours which were found to be 7:45am – 8:45am and 4:30pm – 5:30pm:
 - Site 1 is expected to have a traffic generation of:
 - 76 vehicle trips per hour during the AM peak period (26 in, 50 out); and
 - 74 vehicle trips per hour during the PM peak period (48 in, 26 out).
 - Site 2 is expected to have a traffic generation of:
 - 60 vehicle trips per hour during the AM peak period (18 in, 42 out); and
 - 50 vehicle trips per hour during the PM peak period (36 in, 14 out).



- Site 3 is expected to have a traffic generation of:
 - 49 vehicle trips per hour during the AM peak period (17 in, 32 out); and
 - 35 vehicle trips per hour during the PM peak period (25 in, 10 out).
- ② The existing and existing + development scenarios were modelled using SIDRA Intersection to determine the impact of the additional traffic generation on the local road network. It was found that both scenarios were operating unsatisfactorily. Therefore, further analysis was conducted to propose improvement to the intersections.

The improvements proposed are to modify timing cycle and phase sequence for both intersections, which was shown to have significantly improved average delay and Level of Service. Therefore, the modifications are recommended to be adopted to improve the current operation of the intersections.

- ② The vehicular access requirements for each development have been provided to ensure the concept developments comply with AS2890.1 (2004).

It is therefore concluded that the concept developments are supportable on traffic planning grounds and would operate satisfactorily.



Appendix A

Photographic Record



View looking east along Parkes Street toward sites 2 and 3.



View looking west across Harris Street at site 1.





View looking west across Harris Street at site 2.



View looking north west across the intersection of Parkes Street and Harris Street at site 2.





View looking west along Parkes Street at its intersection with Harris Street.



View looking north along Harris Street at its intersection with Parkes Street.





View looking east along Parkes Street at its intersection with Wigram Street.



View looking south along Wigram Street at its intersection with Parkes Street.



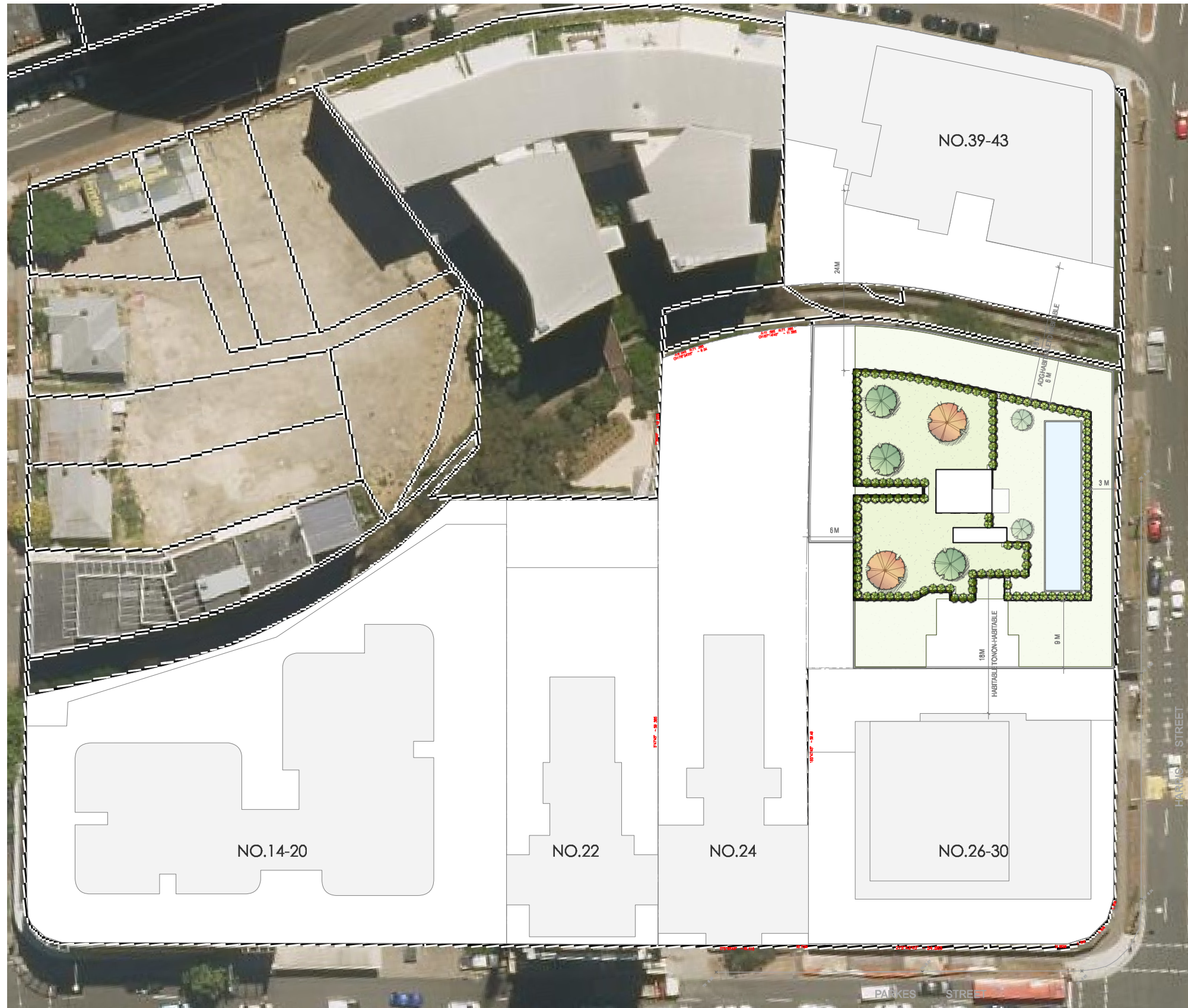


Appendix B

Reduced Plans

ALEKSANDAR
PROJECTS

PROPOSAL
SITE PLAN



NOTE:

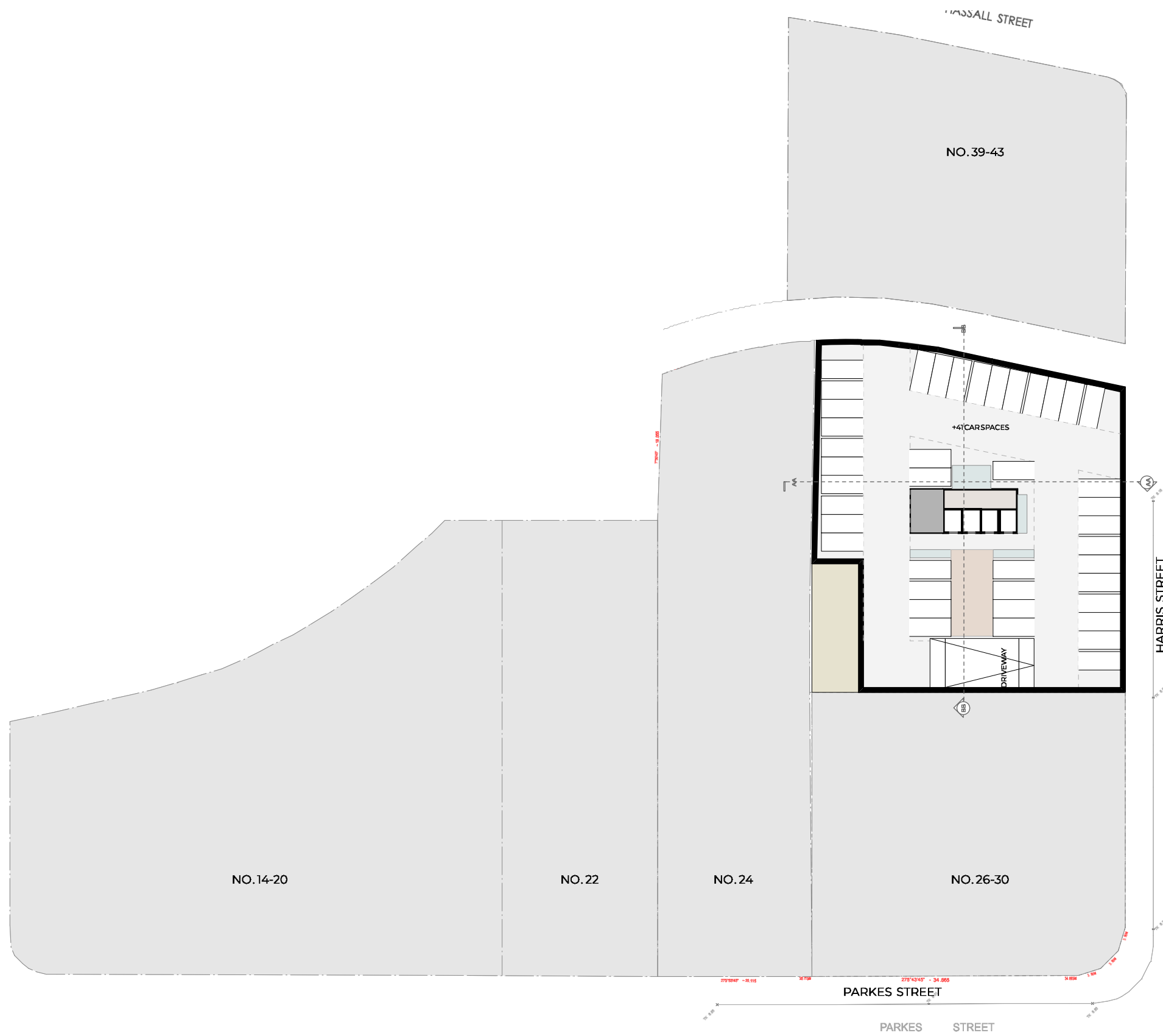
| | |
|----------------------------------|-----------------------|
| COMMUNAL OPEN SPACE CALCULATION: | |
| LEVEL 4: | 440 M2 |
| LEVEL 35: | 260 M2 |
| TOTAL: | 700 M2 (39.4%) |
| | min. 25% required |
| 2 HOURS SOLAR | 410 M2 (58.5%) |
| | min. 50% required |






KEY

- SUBJECT SITE BOUNDARY
- PROPOSED BUILT FORM ADJACENT SITES
- RETAIL
- COMMERCIAL
- 1 BEDROOM
- 2 BEDROOM
- 2 BEDROOM (SPLIT LEVEL)
- 3 BEDROOM
- 4 BEDROOM
- COMMUNAL AREAS
- PROPOSED HABITABLE ROOMS



SCALE 1:600



- KEY**
-  FIRE STAIR
 -  SERVICE / PLANT ROOMS
 -  MOTOBIKE / BIKE PARKING
 -  STORAGE
 -  DEEP SOIL

 SCALE 1:600



NOTE:

CAR SPACES CALCULATION:

| | |
|-------------------------|-----------------|
| TYPICAL BASEMENT LEVEL: | 41 CARS |
| BASEMENT 01: | 38 CARS |
| TOTAL: | 183 CARS |
| | 52 BIKES |

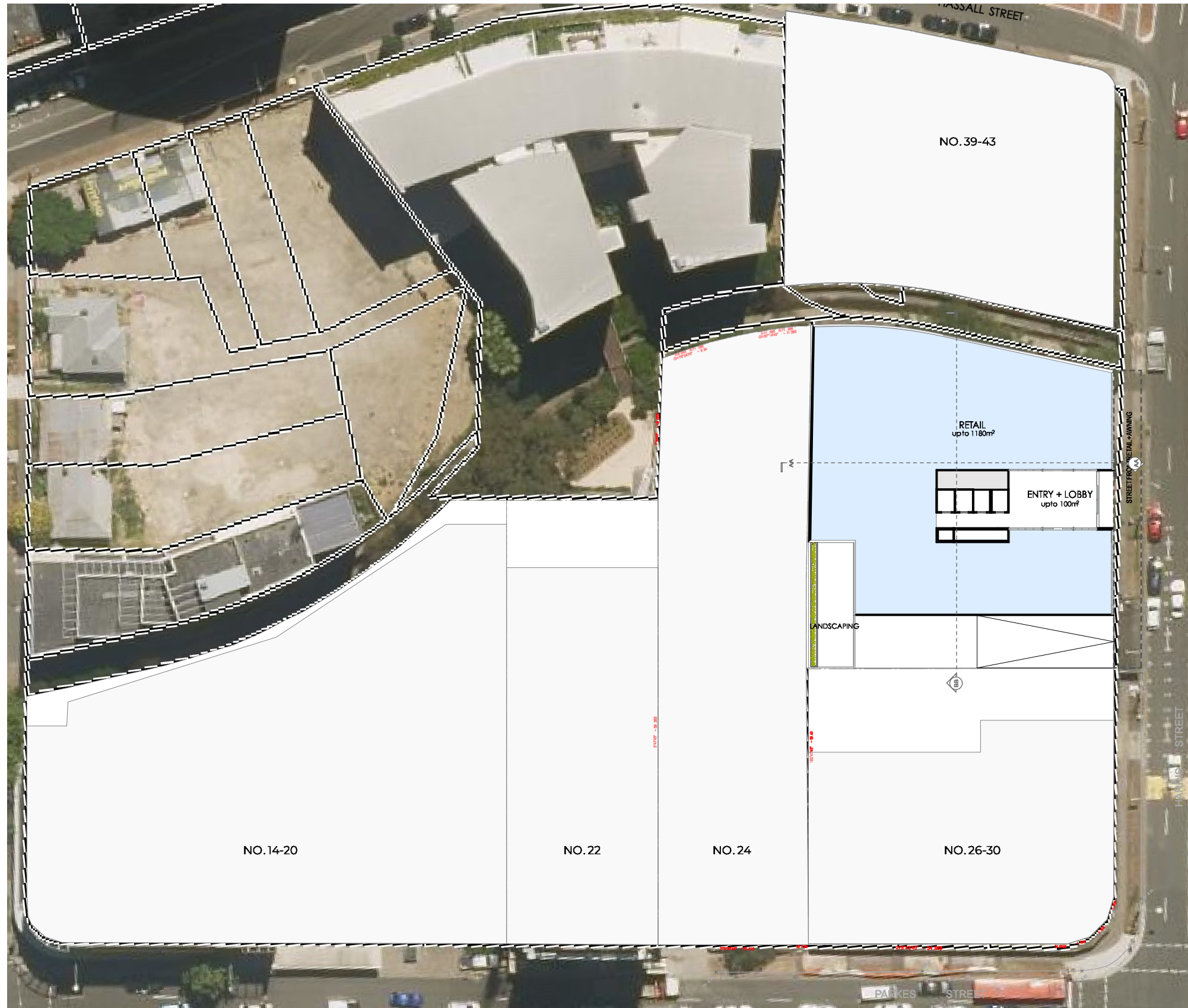
KEY

- FIRE STAIR
- SERVICE / PLANT ROOMS
- MOTOBIKE / BIKE PARKING
- STORAGE
- DEEP SOIL



SCALE 1:600

PROPOSAL
GROUND FLOOR PLAN



KEY

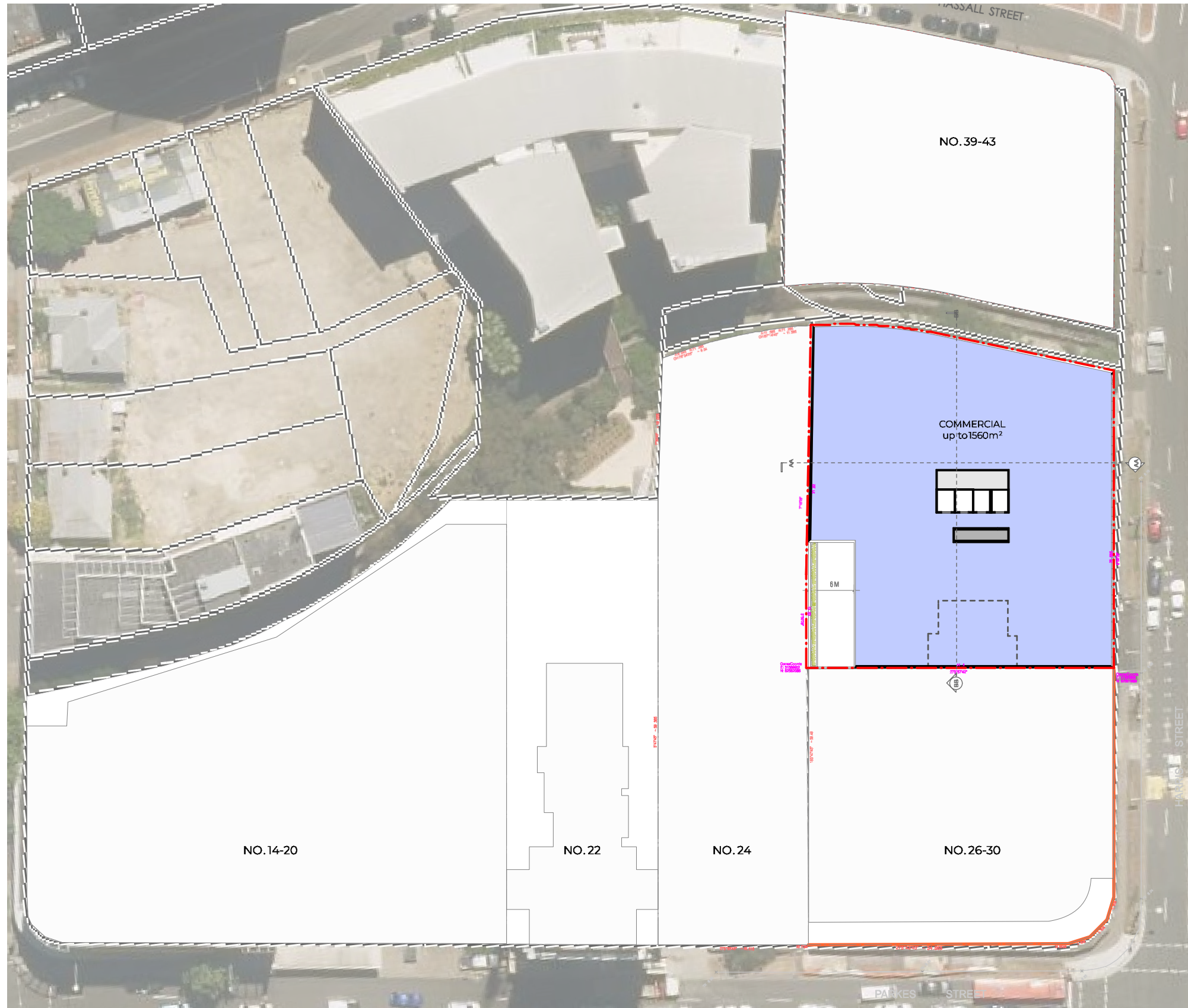
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- PROPOSED BUILT FORM ADJACENT SITES
- RETAIL
- COMMERCIAL
- 1 BEDROOM
- 2 BEDROOM
- 2 BEDROOM (SPLIT LEVEL)
- 3 BEDROOM
- 4 BEDROOM
- COMMUNAL AREAS
- PROPOSED HABITABLE ROOMS



SCALE 1:600

ALEKSANDAR
PROJECTS

PROPOSAL
PODIUM FLOOR PLAN



KEY

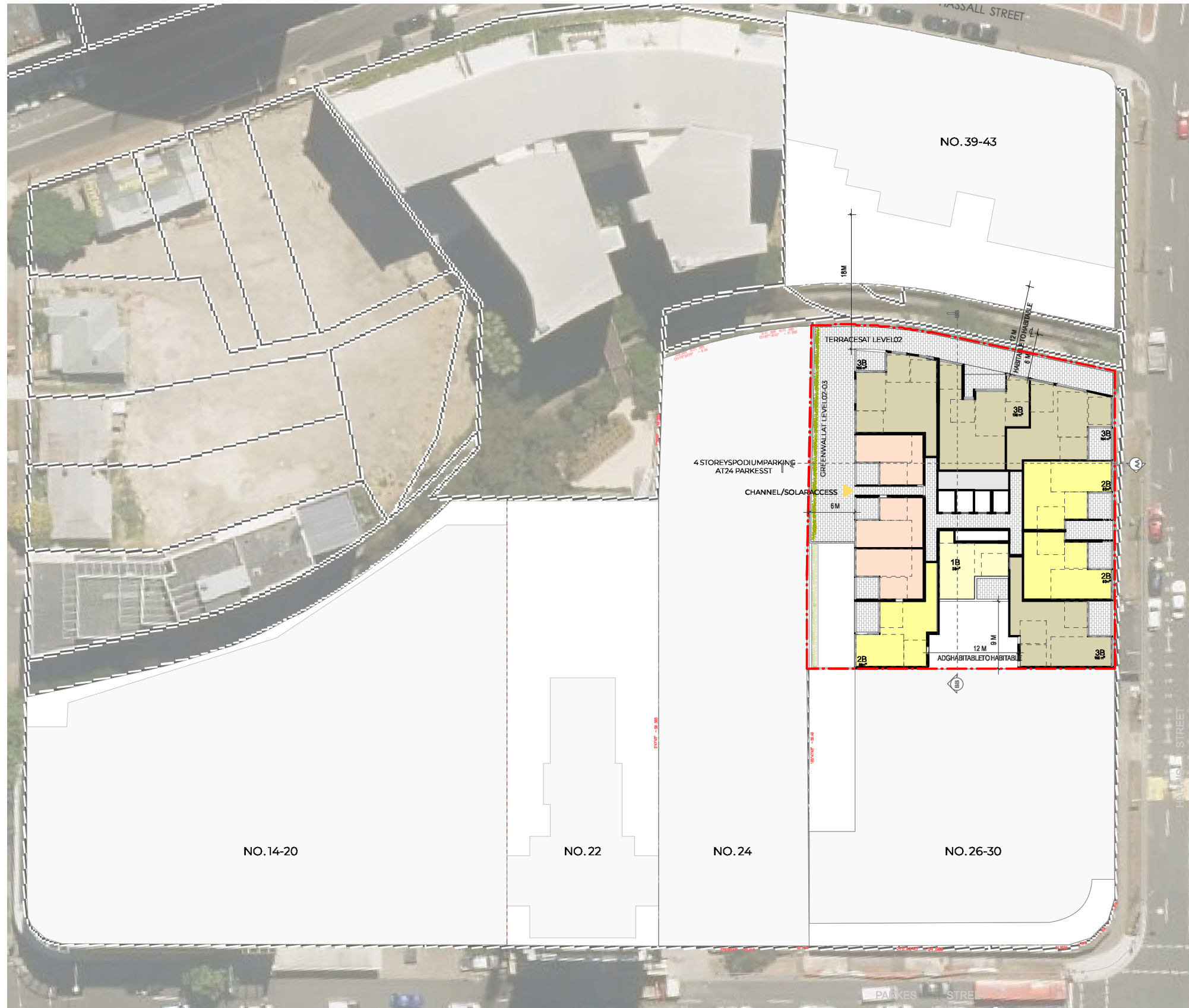
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- PROPOSED BUILT FORM ADJACENT SITES
- RETAIL
- COMMERCIAL
- 1 BEDROOM
- 2 BEDROOM
- 2 BEDROOM (SPLIT LEVEL)
- 3 BEDROOM
- 4 BEDROOM
- COMMUNAL AREAS
- PROPOSED HABITABLE ROOMS



SCALE 1:600

ALEKSANDAR
PROJECTS

PROPOSAL
L2-3 FLOOR PLAN



NOTE:

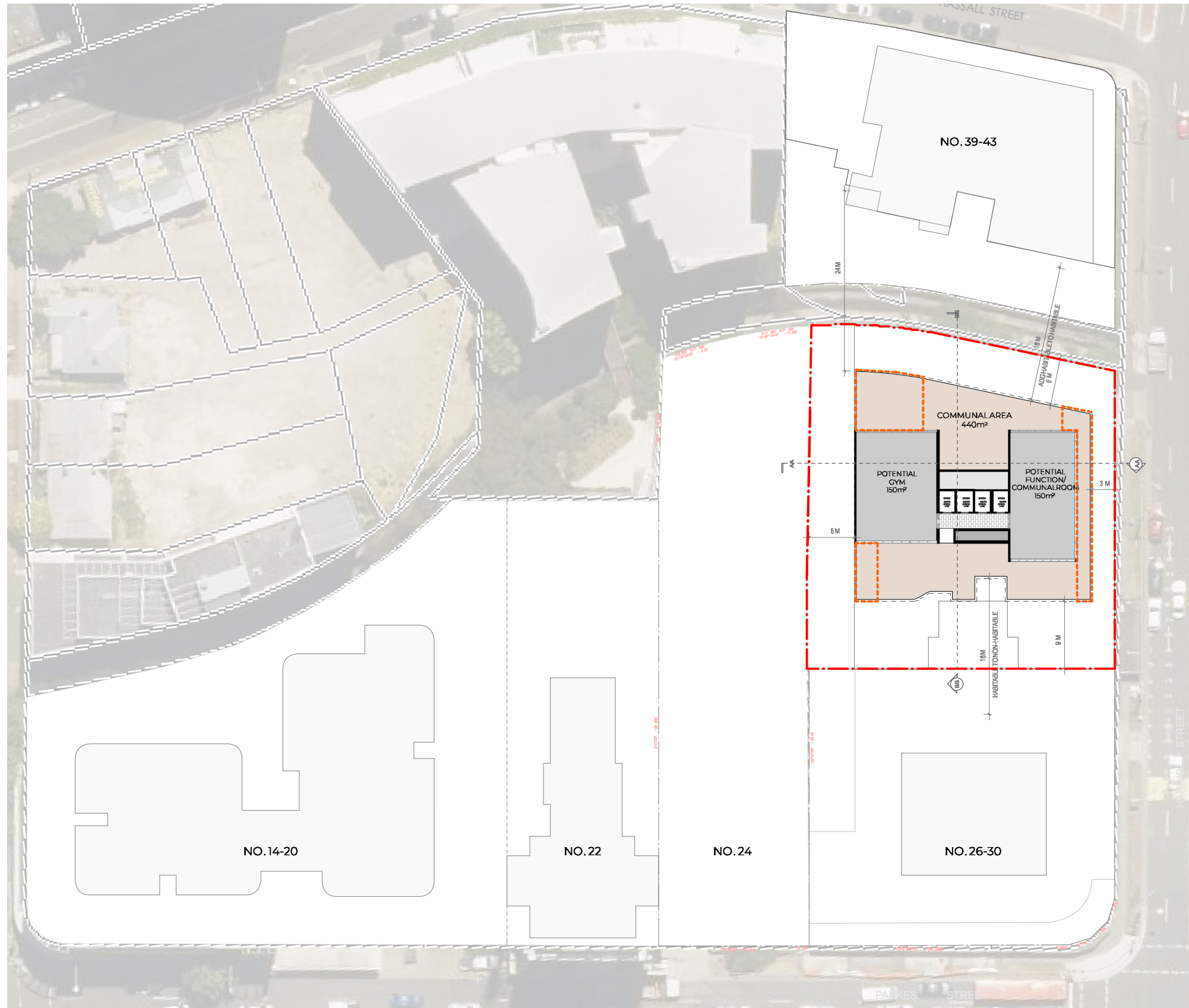
SOLAR + PRIVACY SCREENINGS ARE TO BE DESIGNED AT DA STAGE

KEY

- SUBJECT SITE BOUNDARY
- PROPOSED BUILT FORM ADJACENT SITES
- RETAIL
- COMMERCIAL
- 1 BEDROOM
- 2 BEDROOM
- 2 BEDROOM (SPLIT LEVEL)
- 3 BEDROOM
- 4 BEDROOM
- COMMUNAL AREAS
- PROPOSED HABITABLE ROOMS



SCALE 1:600



NOTE:

COMMUNAL AREA: 440 M2
COS RECEIVING 2 HOURS SOLAR: 150 M2

KEY

- SUBJECT SITE BOUNDARY
- PROPOSED BUILT FORM ADJACENT SITES
- RETAIL
- COMMERCIAL
- 1 BEDROOM
- 2 BEDROOM
- 2 BEDROOM (SPLIT LEVEL)
- 3 BEDROOM
- 4 BEDROOM
- COMMUNAL AREAS
- PROPOSED HABITABLE ROOMS
- COMMUNAL AREAS RECEIVING 2HR SOLAR



SCALE 1:600





KEY

- SUBJECT SITE BOUNDARY
- PROPOSED BUILT FORM ADJACENT SITES
- RETAIL
- COMMERCIAL
- 1 BEDROOM
- 2 BEDROOM
- 2 BEDROOM (SPLIT LEVEL)
- 3 BEDROOM
- 4 BEDROOM
- COMMUNAL AREAS
- PROPOSED HABITABLE ROOMS



SCALE 1:600



NOTE:

COMMUNAL AREA: 260 M2
COS RECEIVING 2 HOURS SOLAR: 260 M2

KEY

- SUBJECT SITE BOUNDARY
- PROPOSED BUILT FORM ADJACENT SITES
- RETAIL
- COMMERCIAL
- 1 BEDROOM
- 2 BEDROOM
- 2 BEDROOM (SPLIT LEVEL)
- 3 BEDROOM
- 4 BEDROOM
- COMMUNAL AREAS
- PROPOSED HABITABLE ROOMS
- COMMUNAL AREAS RECEIVING 2HR SOLAR



SCALE 1:600

ALEKSANDAR PROJECTS

PROPOSAL YIELD

| SITE AREA | | 1776 M2 | | | | |
|--|--------------|--------------------|------------|------------|----------|------------|
| YIELD | | | | | | |
| LEVEL | GFA (M2) | 1 BED | 2 BED | 3 BED | 4 BED | COS (M2) |
| GF | 1280 | | | | | |
| LEVEL 01 | 1560 | | | | | |
| LEVEL 02 | 990 | 1 | 6 | 4 | | |
| LEVEL 03 | 990 | 1 | 3 | 4 | | |
| LEVEL 04 | 300 | | | | | 440 |
| LEVEL 05 | 650 | 1 | 7 | | | |
| LEVEL 06 | 650 | 1 | 7 | | | |
| LEVEL 07 | 650 | 1 | 7 | | | |
| LEVEL 08 | 650 | 1 | 7 | | | |
| LEVEL 09 | 650 | 1 | 7 | | | |
| LEVEL 10 | 650 | 1 | 7 | | | |
| LEVEL 11 | 650 | 1 | 7 | | | |
| LEVEL 12 | 650 | 1 | 7 | | | |
| LEVEL 13 | 650 | 1 | 7 | | | |
| LEVEL 14 | 650 | 1 | 7 | | | |
| LEVEL 15 | 650 | 1 | 7 | | | |
| LEVEL 16 | 650 | 1 | 7 | | | |
| LEVEL 17 | 650 | 1 | 7 | | | |
| LEVEL 18 | 650 | 1 | 7 | | | |
| LEVEL 19 | 650 | 1 | 7 | | | |
| LEVEL 20 | 650 | 1 | 7 | | | |
| LEVEL 21 | 650 | 1 | 7 | | | |
| LEVEL 22 | 650 | 1 | 7 | | | |
| LEVEL 23 | 650 | 1 | 7 | | | |
| LEVEL 24 | 650 | 1 | 7 | | | |
| LEVEL 25 | 650 | 1 | 7 | | | |
| LEVEL 26 | 650 | 1 | 7 | | | |
| LEVEL 27 | 650 | 1 | 7 | | | |
| LEVEL 28 | 650 | 1 | 7 | | | |
| LEVEL 29 | 650 | 1 | 7 | | | |
| LEVEL 30 | 650 | 1 | 7 | | | |
| LEVEL 31 | 650 | 1 | 7 | | | |
| LEVEL 32 | 645 | | 1 | 5 | | |
| LEVEL 33 | 645 | | 1 | 5 | | |
| LEVEL 34 | 645 | | 1 | 5 | | |
| LEVEL 35 | 400 | | | | 2 | 260 |
| LEVEL 36 | 400 | | | | 2 | |
| LEVEL 37 | 400 | | | | 2 | |
| HEIGHT LIMITED TO 38 STOREYS (AT 3.1M FLR TO FLR) DUE TO OVERSHADOWING OF EXPERIMENT FARM COTTAGE | | | | | | |
| TOTALS | 25805 | 35 | 198 | 23 | 6 | 700 |
| | | 13.4% | 75.6% | 8.8% | 2.3% | 39.4% |
| | | TOTAL UNITS | | 262 | | |
| | | FSR | | 14.53 :1 | | |

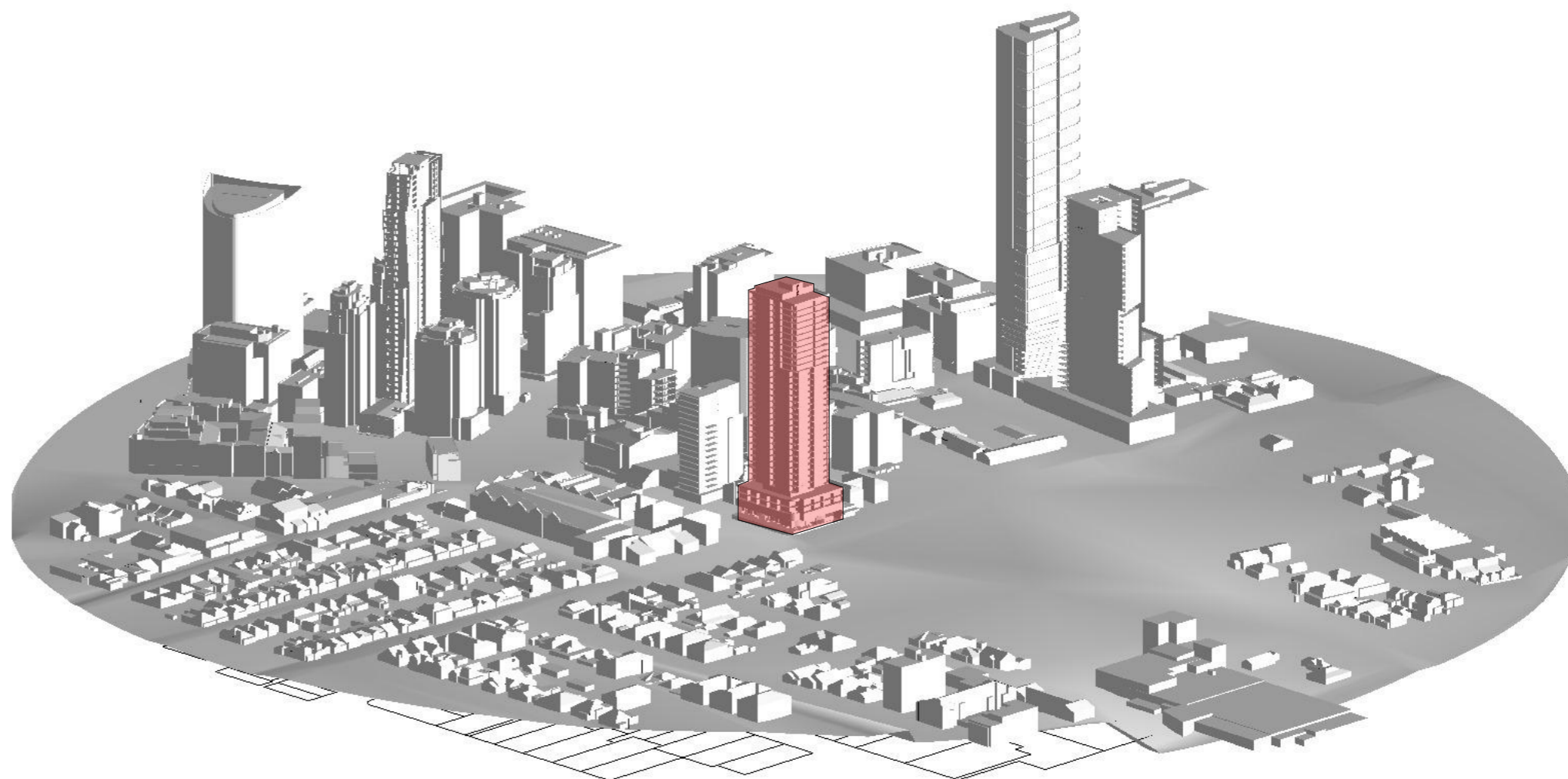
26-30 PARKES STREET, PARRAMATTA

UPDATED PLANNING PROPOSAL

FEBRUARY/2018

PREPARED FOR

PARKES STREET NSW PTY LTD.





1 LOCATION PLAN
1:2000



Project Tourism International Architecture Pty Ltd
Level 10, 263 Clarence Street Sydney NSW 2000
T +61 2 9283 0860 www.ptigroup.com.au ABN 90 050 071 022
Nominated Registered Architect: Peter Israel (reg no. 5064)

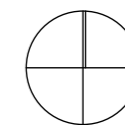
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| P2 | ISSUED FOR REVIEW | AB | 23/10/17 |
| P3 | ISSUED FOR REVIEW | AB | 09/11/17 |
| P4 | ISSUED FOR REVIEW | AB | 26/02/18 |

CLIENT:
PARKES STREET NSW PTY LTD.

PROJECT TITLE:
**26-30 PARKES STREET,
PARRAMATTA**

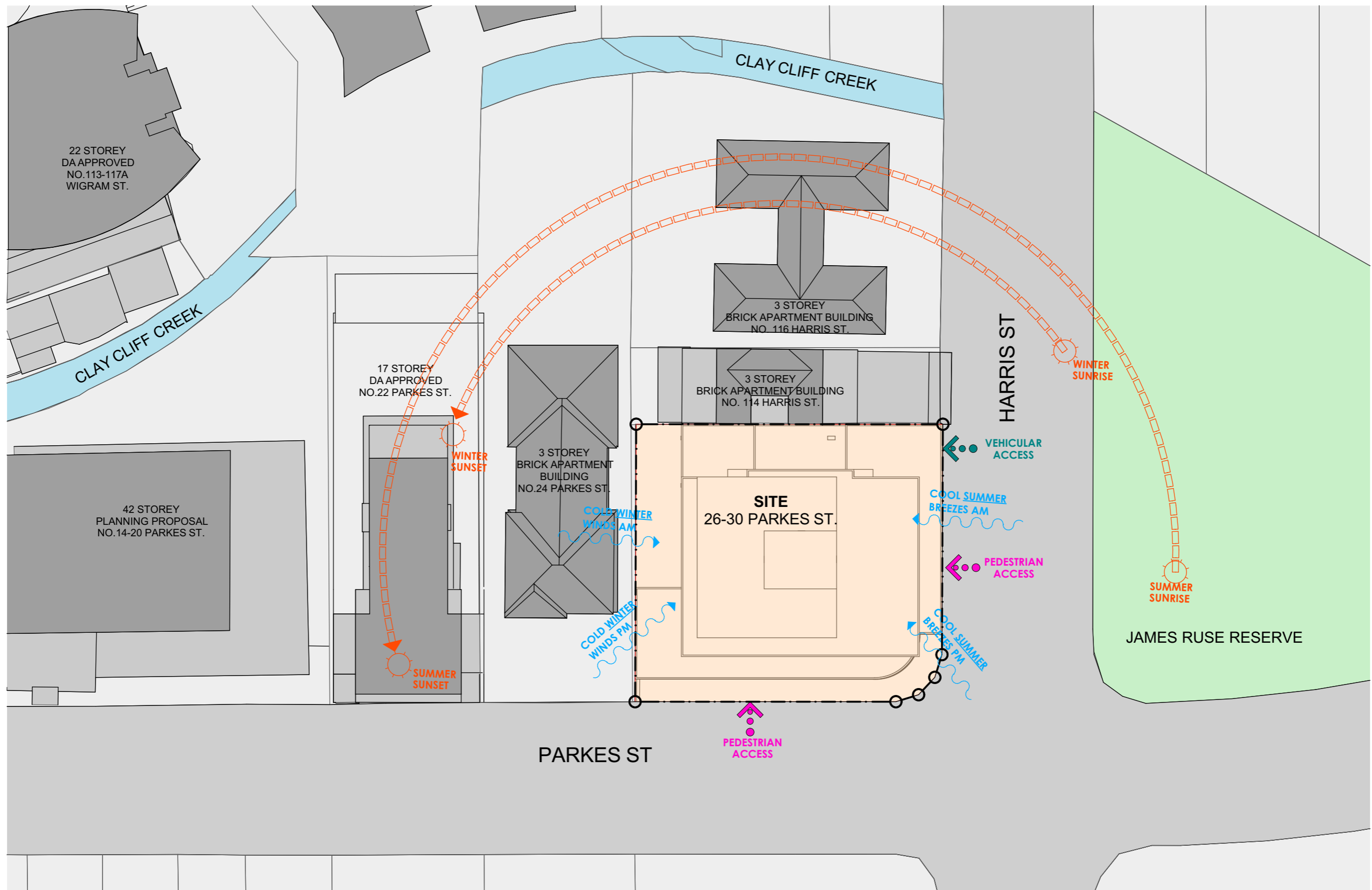
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NORTH POINT:



DRAWN BY: AB
CHECKED BY: PI
SCALE: 1:2000
PROJECT No: P368

SK 01 P4
stage. dwg no. revision



1 SITE ANALYSIS
1:500



Project Tourism International Architecture Pty Ltd
Level 10, 263 Clarence Street Sydney NSW 2000
T +61 2 9283 0860 www.ptigroup.com.au ABN 90 050 071 022
Nominated Registered Architect: Peter Israel (reg no. 5064)

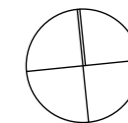
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PROJECT TITLE:
26-30 PARKES STREET,
PARRAMATTA

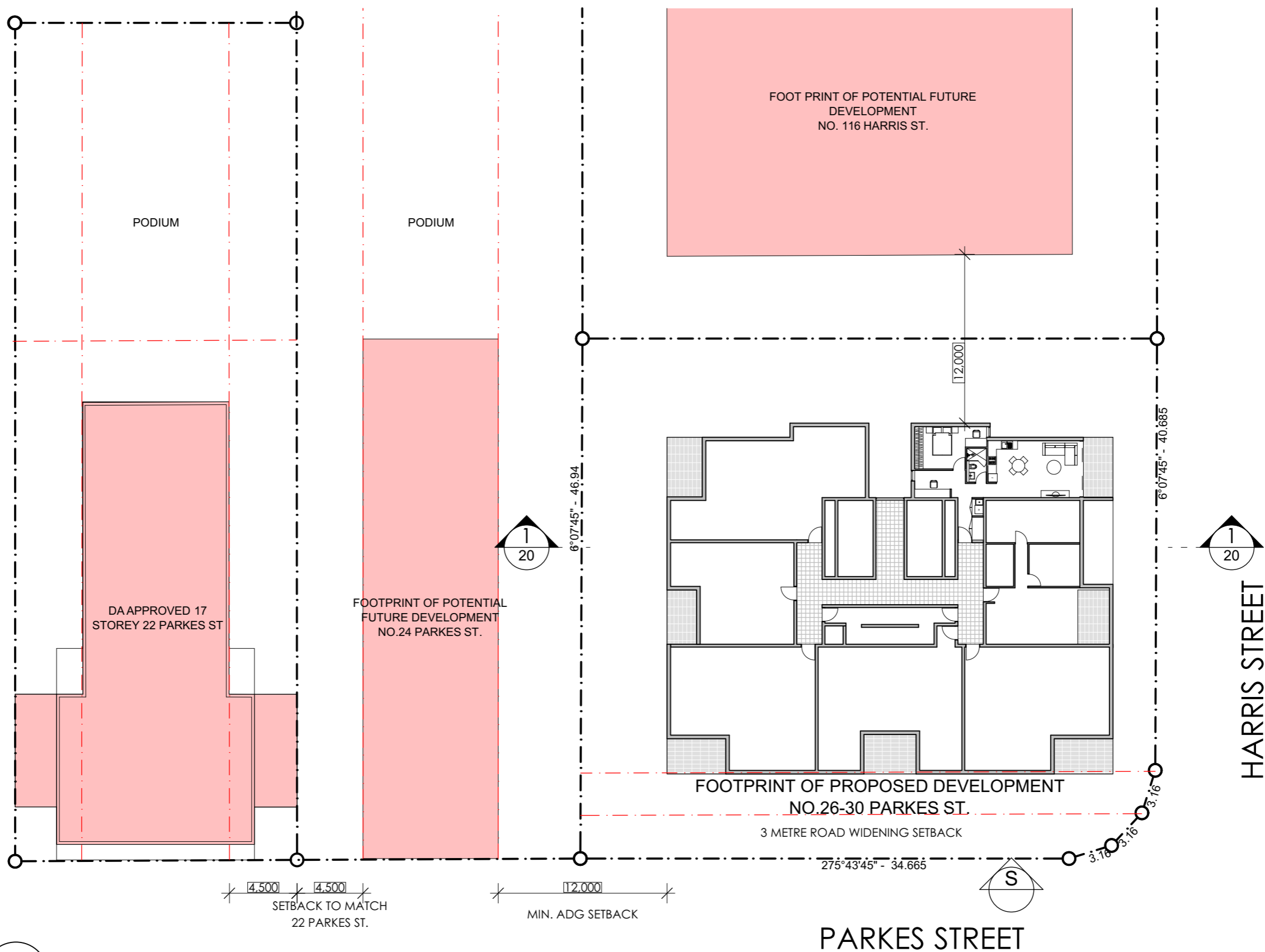
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NORTH POINT:



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PROJECT No: P368

SK 02 P4
stage. dwg no. revision



1 TYPICAL LEVEL-ADJACENT SITES DEVELOPMENT POTENTIAL
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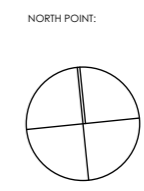


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| P4 | ISSUED FOR REVIEW | AB | 26/02/18 |

CLIENT:
PARKES STREET NSW PTY LTD.

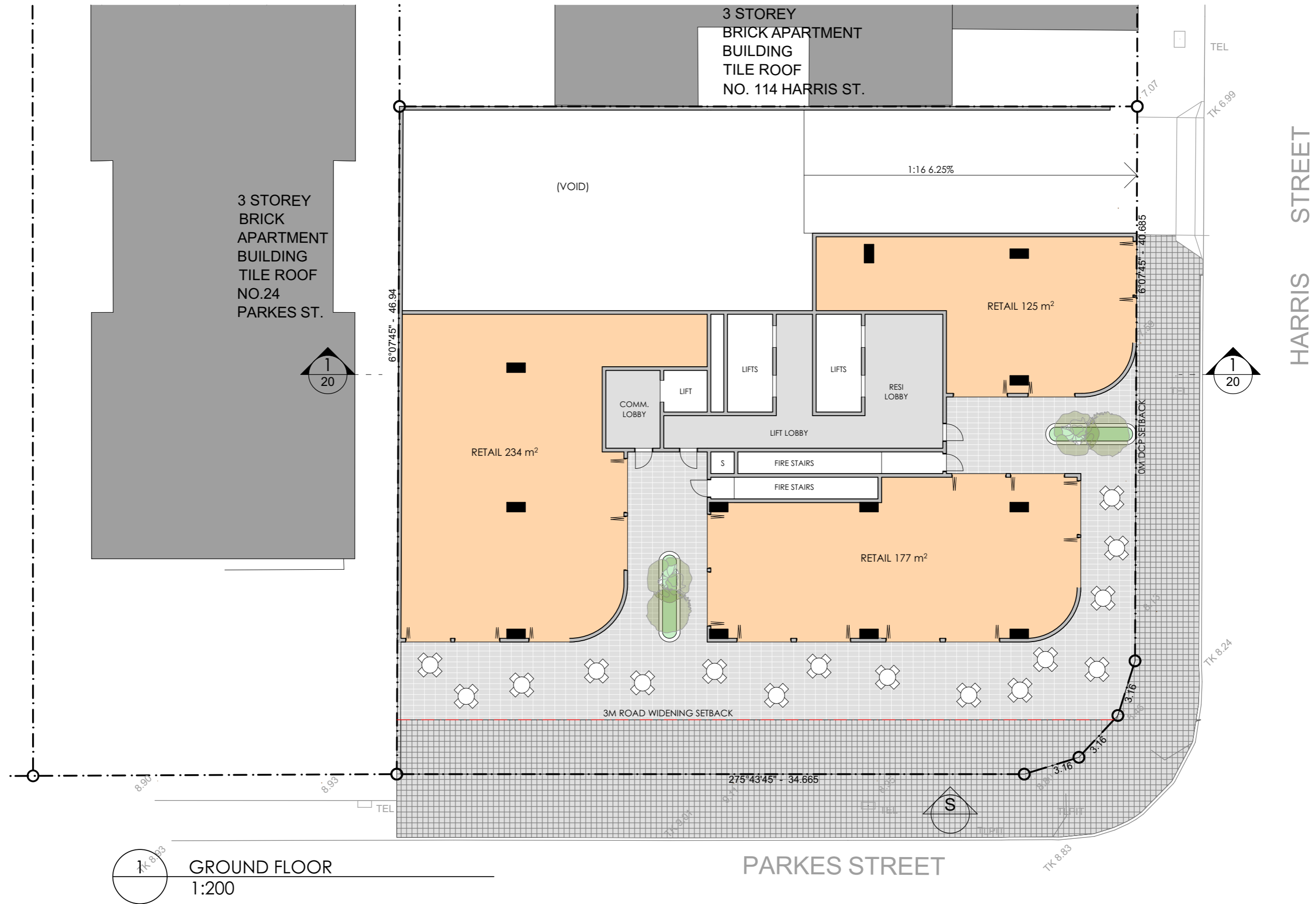
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26-30 PARKES STREET,
PARRAMATTA

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ADJACENT SITES DEVELOPMENT
POTENTIAL



DRAWN BY: AB
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SCALE: 1:300
PROJECT No: P368

SK 03 P4
stage. dwg no. revision



1/200
GROUND FLOOR
1:200



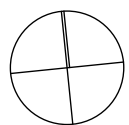
Project Tourism International Architecture Pty Ltd
Level 10, 263 Clarence Street Sydney NSW 2000
T +61 2 9283 0860 www.ptigroup.com.au ABN 90 050 071 022
Nominated Registered Architect: Peter Israel (reg no. 5064)

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|-----|----------------------------|----|----------|
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| P2 | ISSUED FOR REVIEW | AB | 23/10/17 |
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| P4 | ISSUED FOR REVIEW | AB | 26/02/18 |

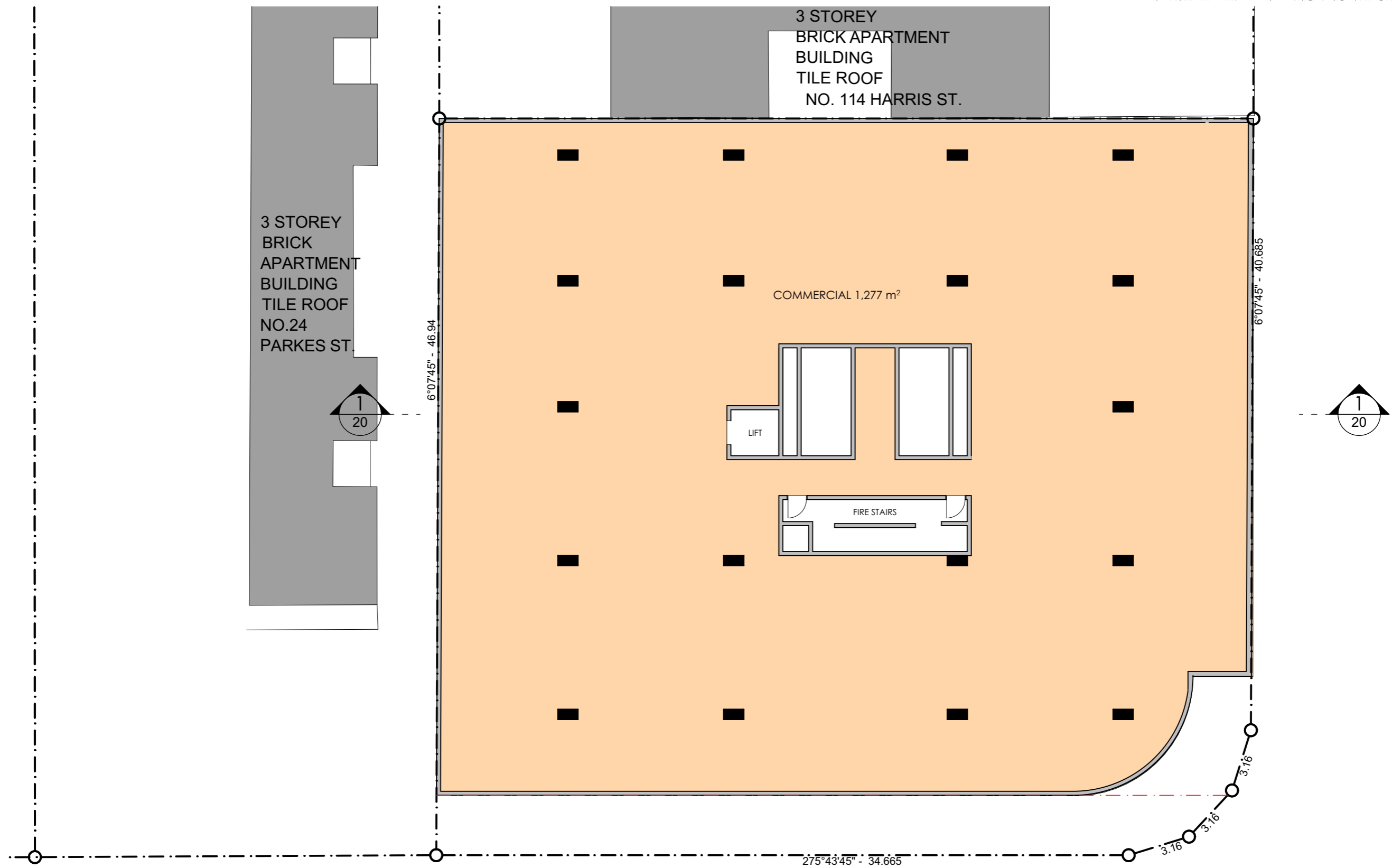
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PARKES STREET NSW PTY LTD.

PROJECT TITLE:
26-30 PARKES STREET,
PARRAMATTA

DRAWING TITLE:
GROUND FLOOR

NORTH POINT:

DRAWN BY: AB
CHECKED BY: PI
SCALE: 1:200
PROJECT No: P368

SK 11 P4
stage. dwg no. revision



1 LEVEL 1 (COMMERCIAL)
1:200



| REV | DESCRIPTION | BY | DATE |
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| P2 | ISSUED FOR REVIEW | AB | 23/10/17 |
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| P4 | ISSUED FOR REVIEW | AB | 26/02/18 |

CLIENT:
PARKES STREET NSW PTY LTD.

PROJECT TITLE:
26-30 PARKES STREET,
PARRAMATTA

DRAWING TITLE:
LEVEL 1 (COMMERCIAL)

NORTH POINT:

DRAWN BY: AB
CHECKED BY: PI
SCALE: 1:200
PROJECT No: P368

SK 12 P4
stage. dwg no. revision



1 LEVEL 2-3 APARTMENT
1:200

PARKES STREET



| REV | DESCRIPTION | BY | DATE |
|-----|----------------------------|----|----------|
| P1 | ISSUED FOR COUNCIL MEETING | AB | 3/10/17 |
| P2 | ISSUED FOR REVIEW | AB | 23/10/17 |
| P3 | ISSUED FOR REVIEW | AB | 09/11/17 |
| P4 | ISSUED FOR REVIEW | AB | 26/02/18 |

CLIENT:
PARKES STREET NSW PTY LTD.

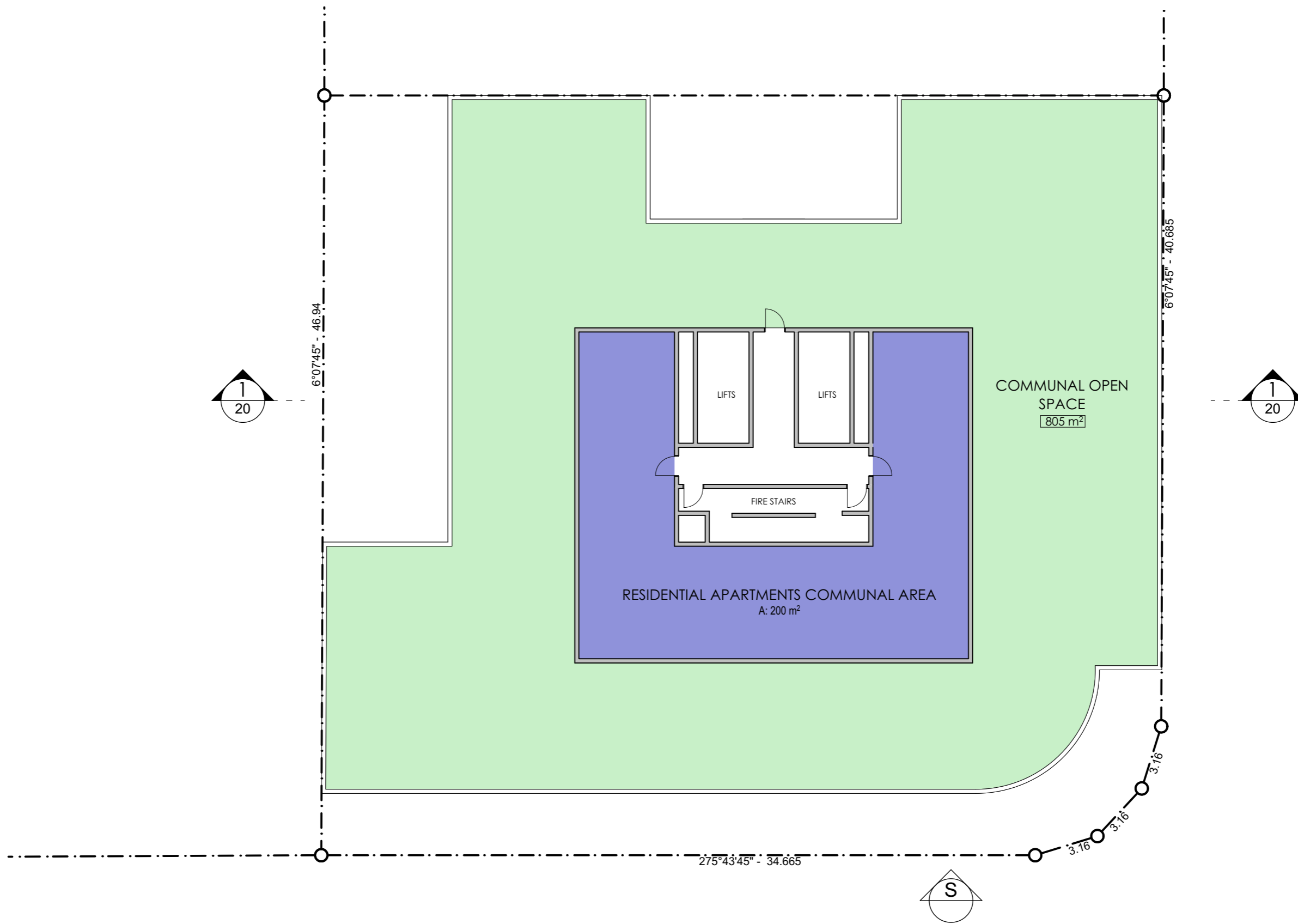
PROJECT TITLE:
26-30 PARKES STREET,
PARRAMATTA

DRAWING TITLE:
LEVEL 2-3 (RESIDENTIAL)

NORTH POINT:

DRAWN BY: AB
CHECKED BY: PI
SCALE: 1:200
PROJECT No: P368

SK 13 P4
stage. dwg no. revision



1 LEVEL 4 (PODIUM)
1:200

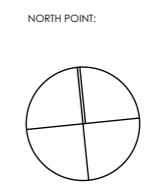


| REV | DESCRIPTION | BY | DATE |
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| P1 | ISSUED FOR COUNCIL MEETING | AB | 3/10/17 |
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| P4 | ISSUED FOR REVIEW | AB | 26/02/18 |

CLIENT:
PARKES STREET NSW PTY LTD.

PROJECT TITLE:
**26-30 PARKES STREET,
PARRAMATTA**

DRAWING TITLE:
LEVEL 4 (PODIUM)



DRAWN BY: AB
CHECKED BY: PI
SCALE: 1:200
PROJECT No: P368

SK 14 P4
stage. dwg no. revision



1 LEVEL 5-24 (TYPICAL APARTMENTS)
1:200

PARKES STREET

HARRIS STREET



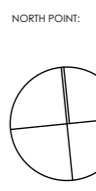
Project Tourism International Architecture Pty Ltd
Level 10, 263 Clarence Street Sydney NSW 2000
T +61 2 9283 0860 www.ptigroup.com.au ABN 90 050 071 022
Nominated Registered Architect: Peter Israel (reg no. 5064)

| REV | DESCRIPTION | BY | DATE |
|-----|----------------------------|----|----------|
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| P4 | ISSUED FOR REVIEW | AB | 26/02/18 |

CLIENT:
PARKES STREET NSW PTY LTD.

PROJECT TITLE:
26-30 PARKES STREET,
PARRAMATTA

DRAWING TITLE:
LEVEL 5-24 TYPICAL RESIDENTIAL
LEVELS



DRAWN BY: AB
CHECKED BY: PI
SCALE: 1:200
PROJECT No: P368

SK 15 P4
stage. dwg no. revision



1 LEVEL 5-24 (RESIDENTIAL OPTION)
1:200



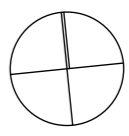
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| P4 | ISSUED FOR REVIEW | AB | 26/02/18 |

CLIENT:
PARKES STREET NSW PTY LTD.

PROJECT TITLE:
26-30 PARKES STREET,
PARRAMATTA

DRAWING TITLE:
LEVEL 5-24 TYPICAL RESIDENTIAL
LEVEL OPTION

NORTH POINT:



DRAWN BY: AB
CHECKED BY: PI
SCALE: 1:200
PROJECT No: P368

SK 16 P4
stage. dwg no. revision



1 LEVEL 25-34 (UPPER LEVEL RESIDENTIAL)
1:200



| REV | DESCRIPTION | BY | DATE |
|-----|----------------------------|----|----------|
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| P4 | ISSUED FOR REVIEW | AB | 26/02/18 |

CLIENT:
PARKES STREET NSW PTY LTD.

PROJECT TITLE:
26-30 PARKES STREET,
PARRAMATTA

DRAWING TITLE:
LEVEL 25-34 TYPICAL RESIDENTIAL
LEVEL

NORTH POINT:

DRAWN BY: AB
CHECKED BY: PI
SCALE: 1:200
PROJECT No: P368

SK 17 P4
stage. dwg no. revision



1 LEVEL 35-FUNCTION ENTRY AND UNITS
1:200

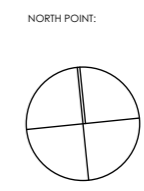


| REV | DESCRIPTION | BY | DATE |
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| P1 | ISSUED FOR COUNCIL MEETING | AB | 3/10/17 |
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CLIENT:
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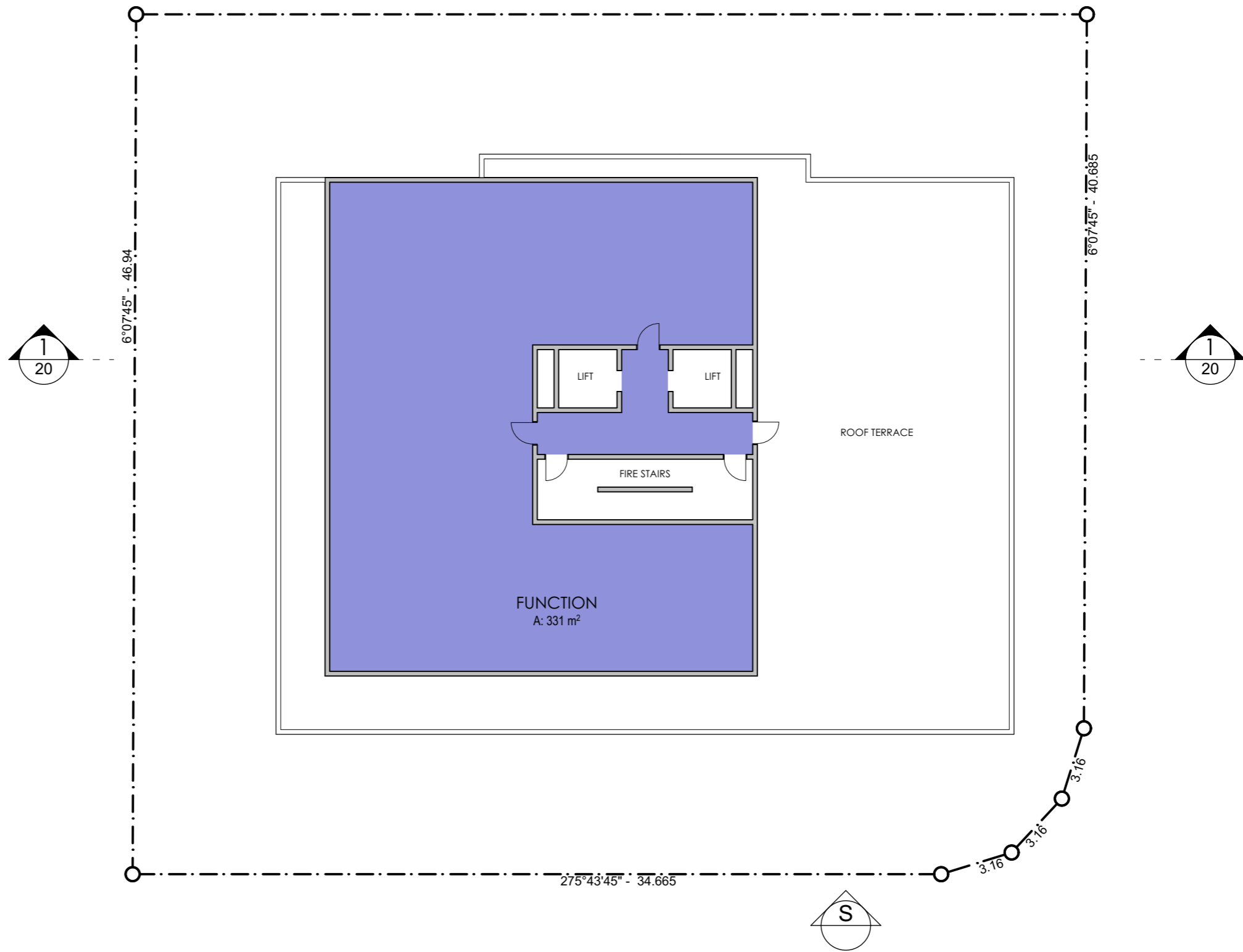
PROJECT TITLE:
26-30 PARKES STREET,
PARRAMATTA

DRAWING TITLE:
LEVEL 35 FUNCTION ENTRY AND
UNITS



DRAWN BY: AB
CHECKED BY: PI
SCALE: 1:200
PROJECT No: P368

SK 18 P4
stage. dwg no. revision



1 LEVEL 36 ROOF FUNCTION
1:200

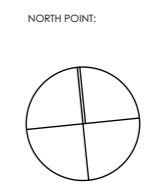


| REV | DESCRIPTION | BY | DATE |
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| P4 | ISSUED FOR REVIEW | AB | 26/02/18 |

CLIENT:
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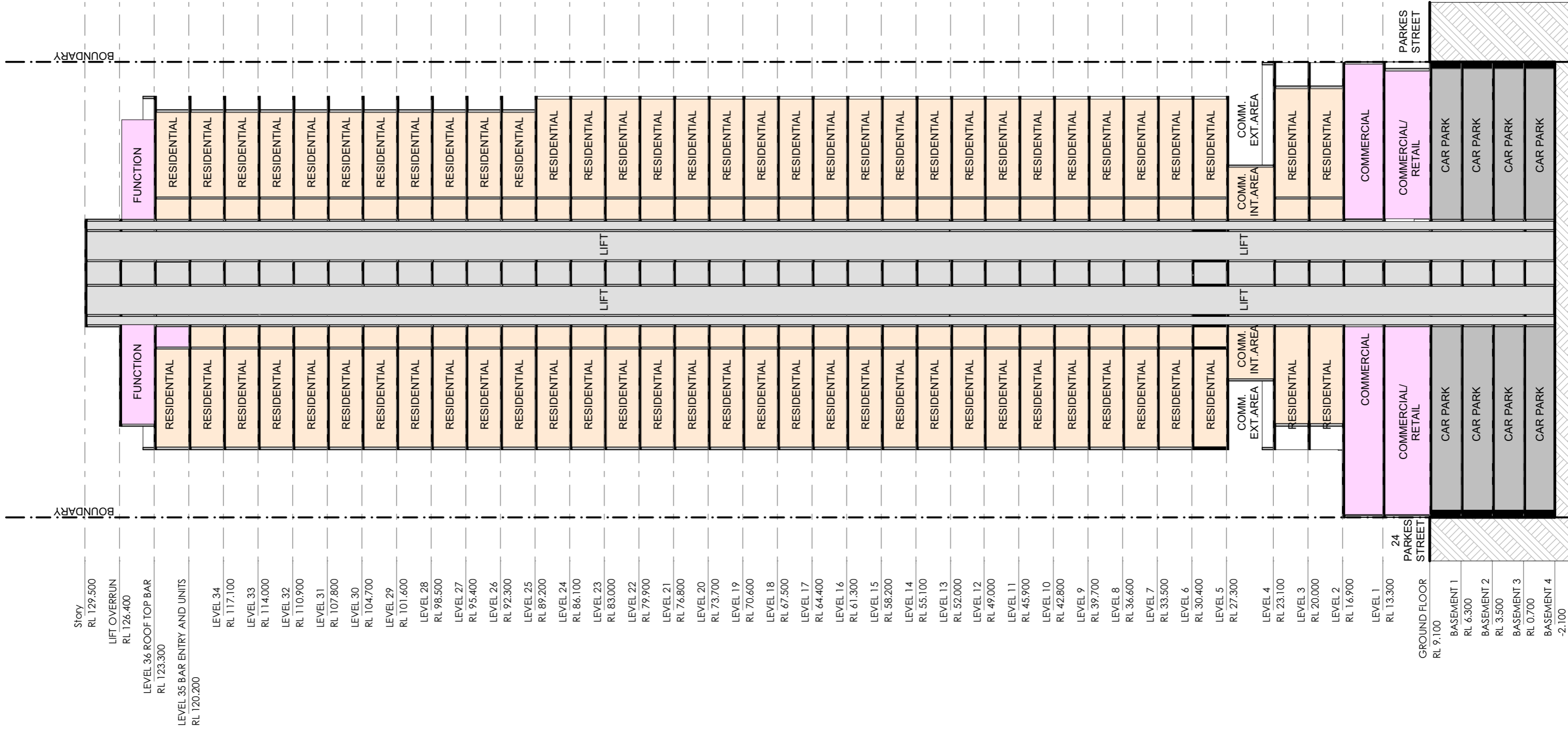
PROJECT TITLE:
26-30 PARKES STREET,
PARRAMATTA

DRAWING TITLE:
LEVELS 36 ROOF TOP FUNCTION



DRAWN BY: AB
CHECKED BY: PI
SCALE: 1:200
PROJECT No: P368

SK 19 P4
stage. dwg no. revision



Story
 RL 129.500
 LIFT OVERRUN
 RL 126.400
 LEVEL 36 ROOF TOP BAR
 RL 123.300
 LEVEL 35 BAR ENTRY AND UNITS
 RL 120.200

LEVEL 34
 RL 117.100
 LEVEL 33
 RL 114.000
 LEVEL 32
 RL 110.900
 LEVEL 31
 RL 107.800
 LEVEL 30
 RL 104.700
 LEVEL 29
 RL 101.600
 LEVEL 28
 RL 98.500
 LEVEL 27
 RL 95.400
 LEVEL 26
 RL 92.300
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 RL 89.200
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 CAR PARK
 CAR PARK

LIFT
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 LIFT

24 PARKES STREET
 GROUND FLOOR
 RL 9.100
 BASEMENT 1
 RL 6.300
 BASEMENT 2
 RL 3.500
 BASEMENT 3
 RL 0.700
 BASEMENT 4
 -2.100



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 Nominated Registered Architect: Peter Israel (reg no. 5064)

| REV | DESCRIPTION | BY | DATE |
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| P4 | ISSUED FOR REVIEW | AB | 26/02/18 |

CLIENT:
PARKES STREET NSW PTY LTD.

PROJECT TITLE:
**26-30 PARKES STREET,
 PARRAMATTA**

DRAWING TITLE:
SECTIONS

NORTH POINT:

DRAWN BY: AB
 CHECKED BY: PI
 SCALE: 1:350
 PROJECT No: P368

stage: **SK**
 revision: **20** **P4**

CONCEPT DESIGN

Proposed Option SK-8
24 Parkes Street Parramatta NSW 2150



| | |
|-------------------------------|-----------------------|
| Site Area 1631 m ² | |
| COMMERCIAL - | |
| Ground -L3 | 1,630 m ² |
| | FSR 1:1 |
| RESIDENTIAL | GFA |
| Typical L5 - L18 | |
| L20-L41 | 18,406 |
| L43-L55 | 350 |
| Penthouse L56 | 18,756 m ² |
| | FSR 11.5:1 |

| ISSUE | AMENDMENT | DATE | DRAWN/CHECKED |
|-------|-----------|------|---------------|
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 www.zhinar.com.au / w
 28 495 869 790 / abn

PROJECT STATUS:
 Option SK-8

PROJECT NAME:
 Concept Design
 24 Parkes Street
 Parramatta NSW 2150

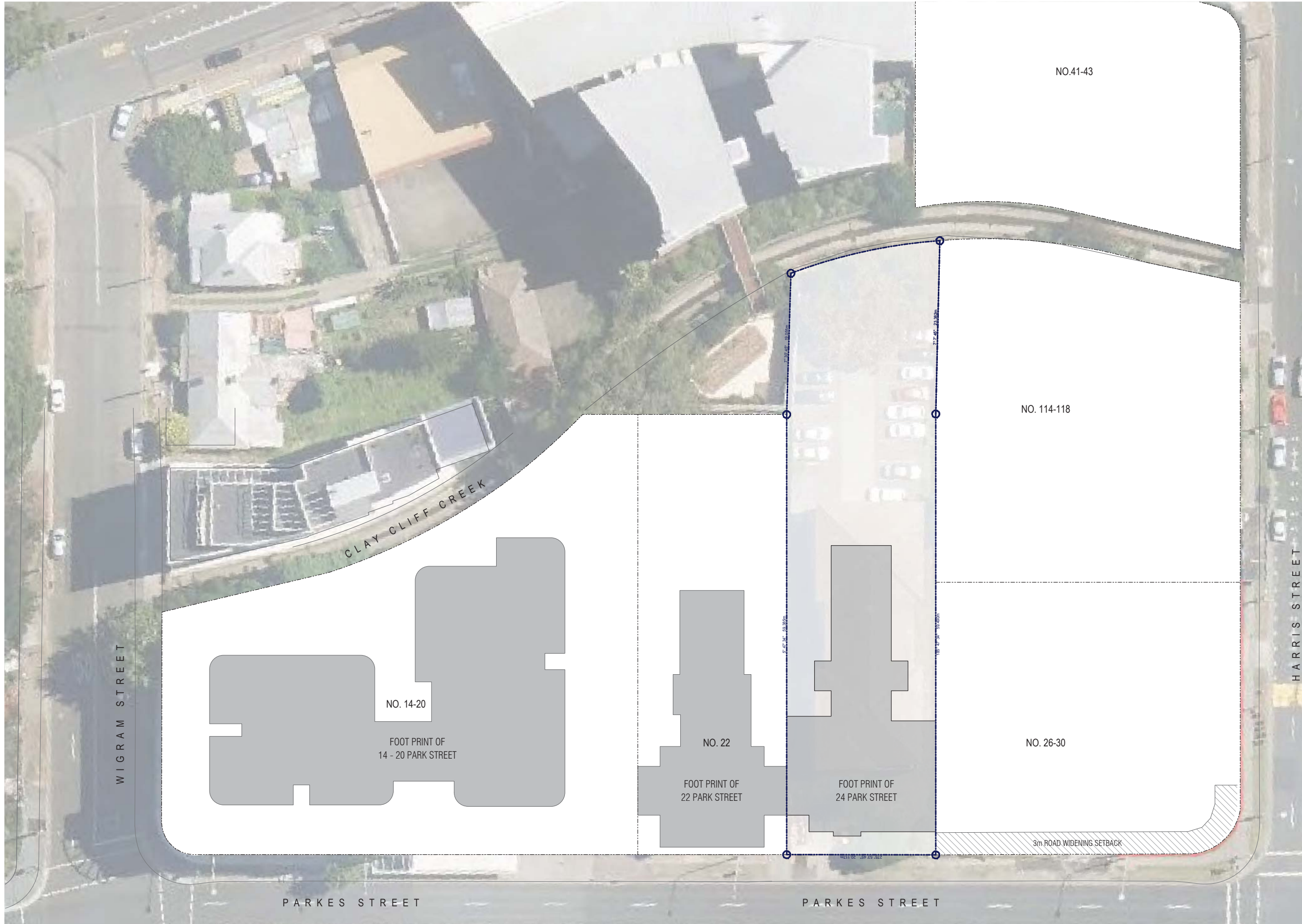
L.G.A.: Parramatta City Council
NORTH:



SHEET TITLE:
 Cover Sheet

DESIGNED: AHM **DRAWN:** AH **COMMENCED:** February 2016 **SCALE:** **PRINT:** A3 SHEET

08486 **SK-8 01** **8**
JOB No. DRAWING No. ISSUE



| ISSUE | AMENDMENT | DATE | DRAWN | CHECKED |
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PROJECT STATUS:
 Option SK-8

PROJECT NAME:
 Concept Design
 24 Parkes Street
 Parramatta NSW 2150

L.G.A.: Parramatta City Council



SHEET TITLE:
 Potential Development Site Plan

DESIGNED: AHM **DRAWN:** AH **COMMENCED:** February 2016 **SCALE:** **PRINT:** A3 SHEET

JOB No.: 08486 **DRAWING No.:** SK-8 02 **ISSUE:** 8

Potential Development Site Plan
 Scale @A1 - 1:250



| ISSUE | AMENDMENT | DATE | DRAWN | CHECKED |
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PROJECT STATUS:
 Option SK-8

PROJECT NAME:
 Concept Design
 24 Parkes Street
 Parramatta NSW 2150

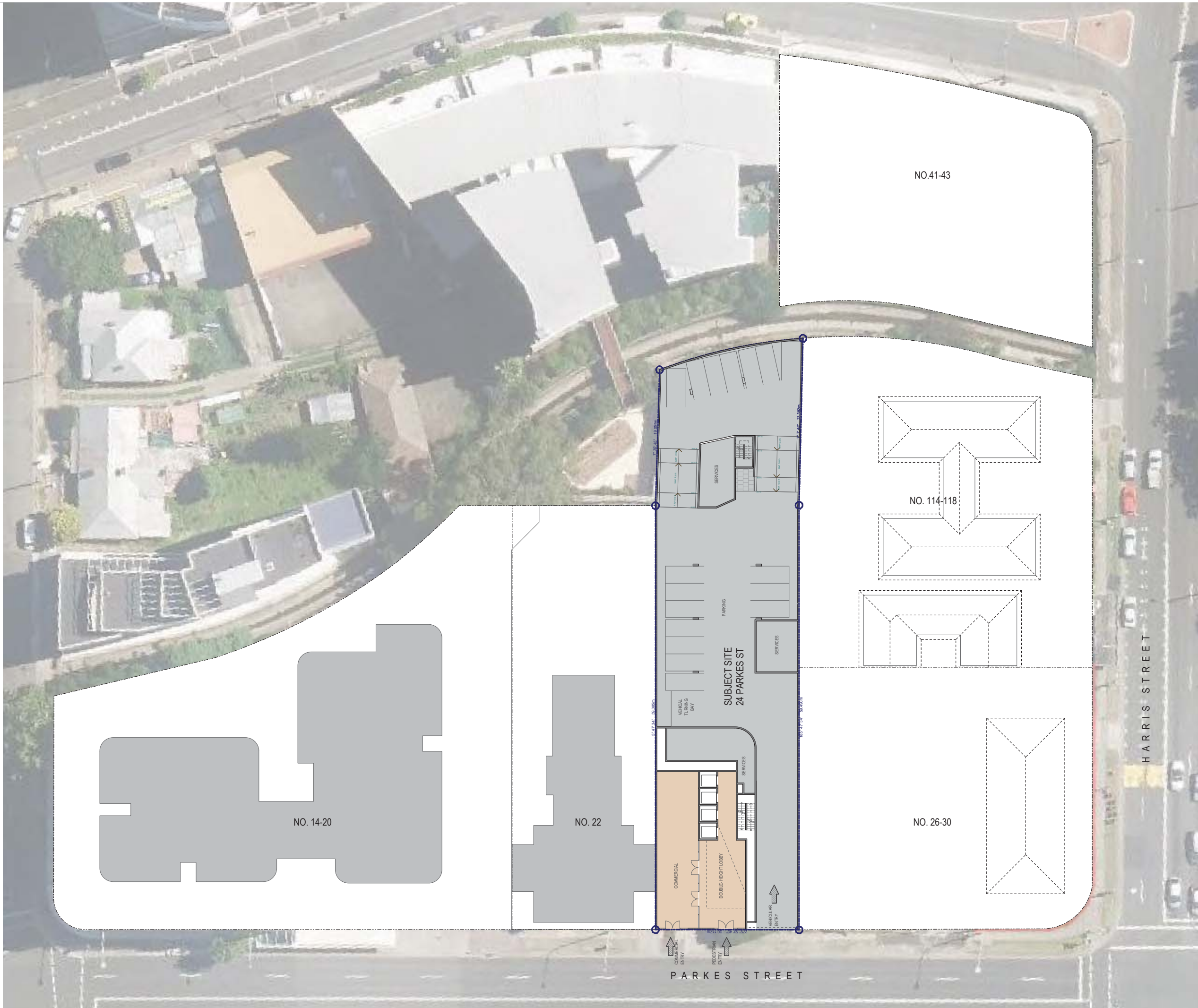
L.G.A.: Parramatta City Council
NORTH:



SHEET TITLE:
 Typical Basements

DESIGNED: AHM **DRAWN:** AH **COMMENCED:** February 2016 **SCALE:** **PRINT:** A3 SHEET

08486 **SK-8 03** **8**
 JOB No. DRAWING No. ISSUE



| ISSUE | AMENDMENT | DATE | DRAWN | CHECKED |
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PROJECT STATUS:
 Option SK-8

PROJECT NAME:
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 24 Parkes Street
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NORTH:

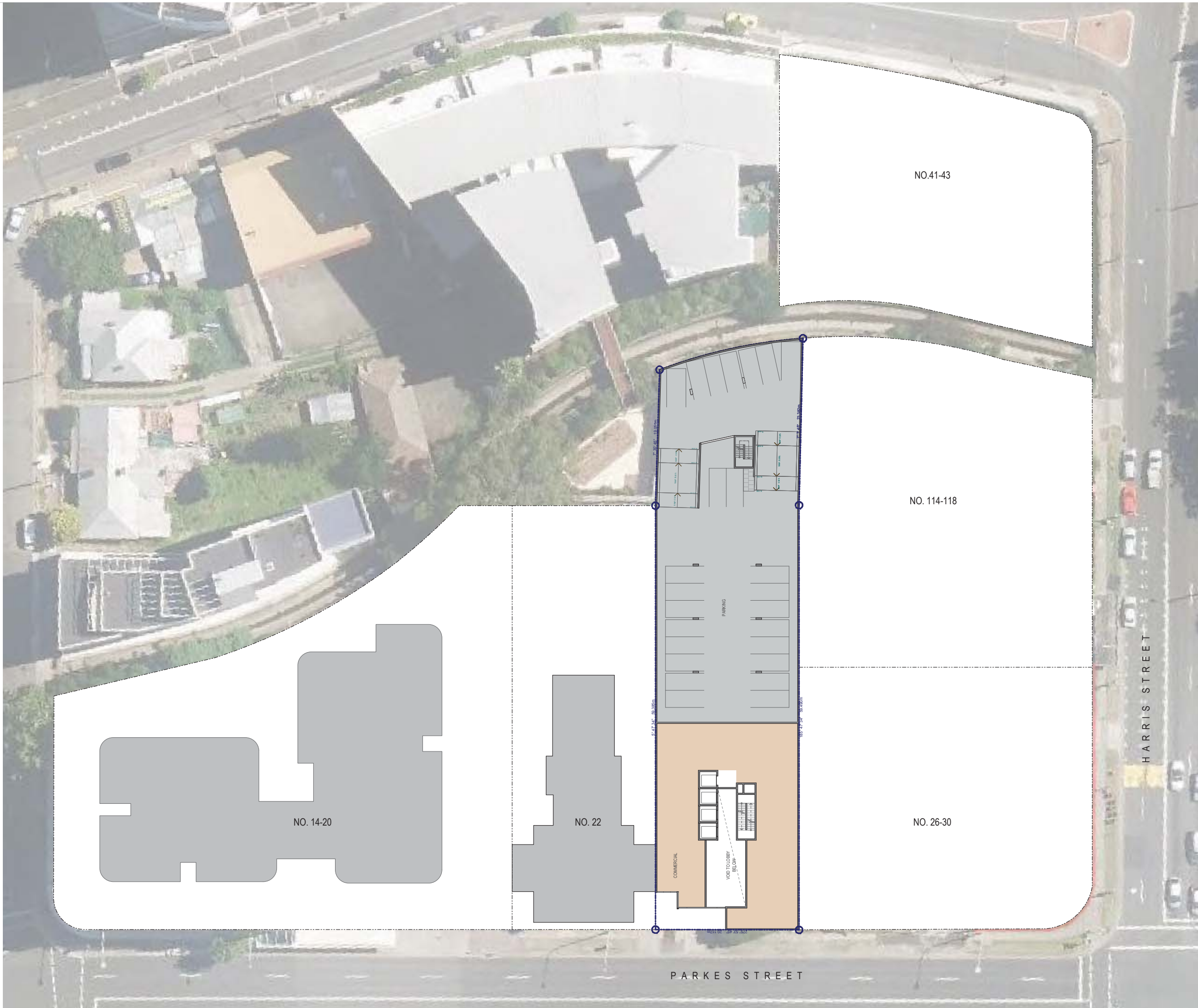


SHEET TITLE:
 Ground - Entry & Commercial

DESIGNED: AHM **DRAWN:** AH **COMMENCED:** February 2016 **SCALE:** 1:500 **PRINT:** A3 SHEET

JOB No.: 08486 **DRAWING No.:** SK-8 04 **ISSUE:** 8

PRINT DATE: Thursday, 10 May 2018 11:59 am



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PROJECT STATUS:
 Option SK-8

PROJECT NAME:
 Concept Design
 24 Parkes Street
 Parramatta NSW 2150

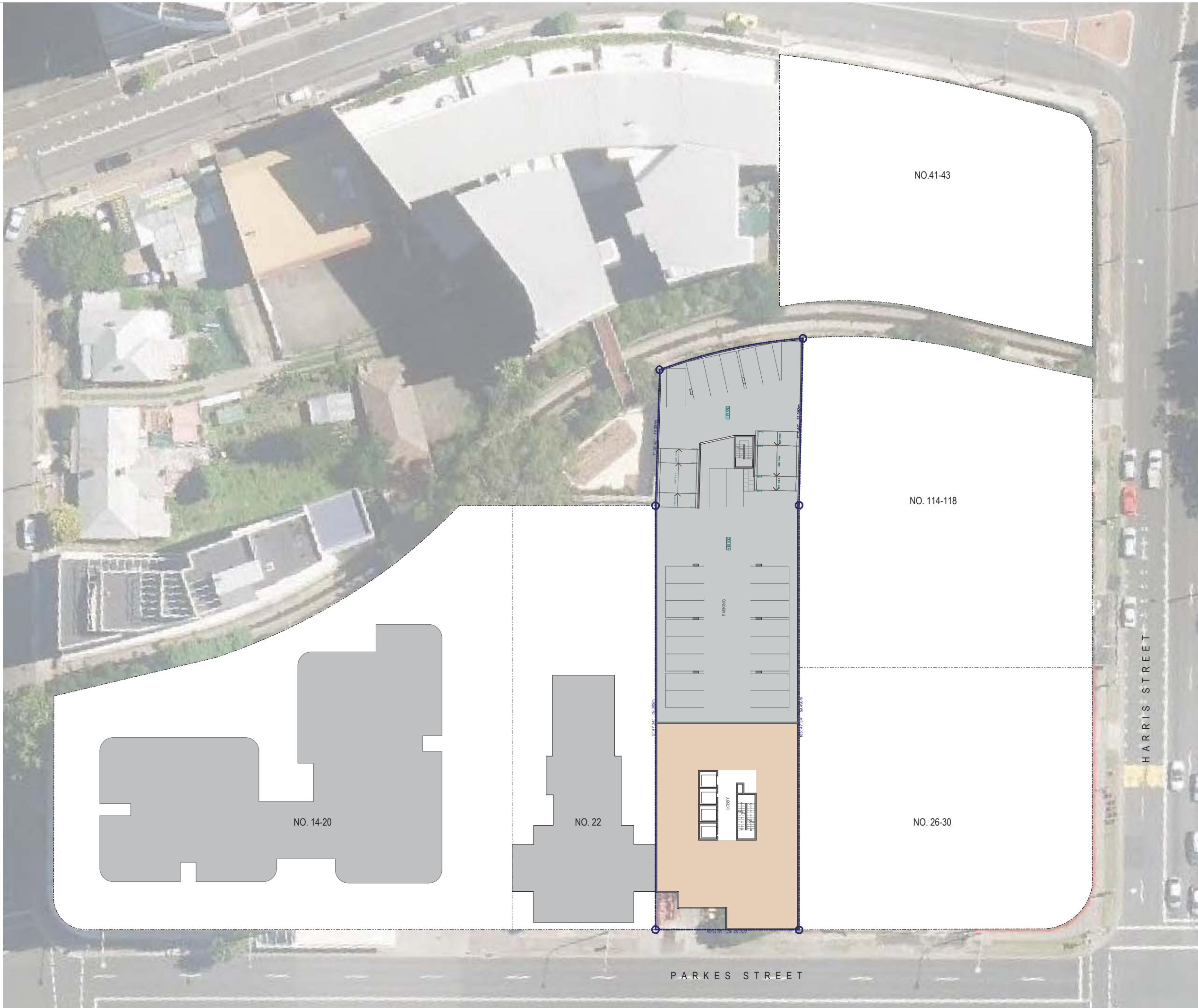
L.G.A.: Parramatta City Council
NORTH:



SHEET TITLE:
 L1 - Commercial

DESIGNED: AHM **DRAWN:** AH **COMMENCED:** February 2016 **SCALE:** 1:500 **PRINT:** A3 SHEET

JOB No.: 08486 **DRAWING No.:** SK-8 05 **ISSUE:** 8



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 Option SK-8

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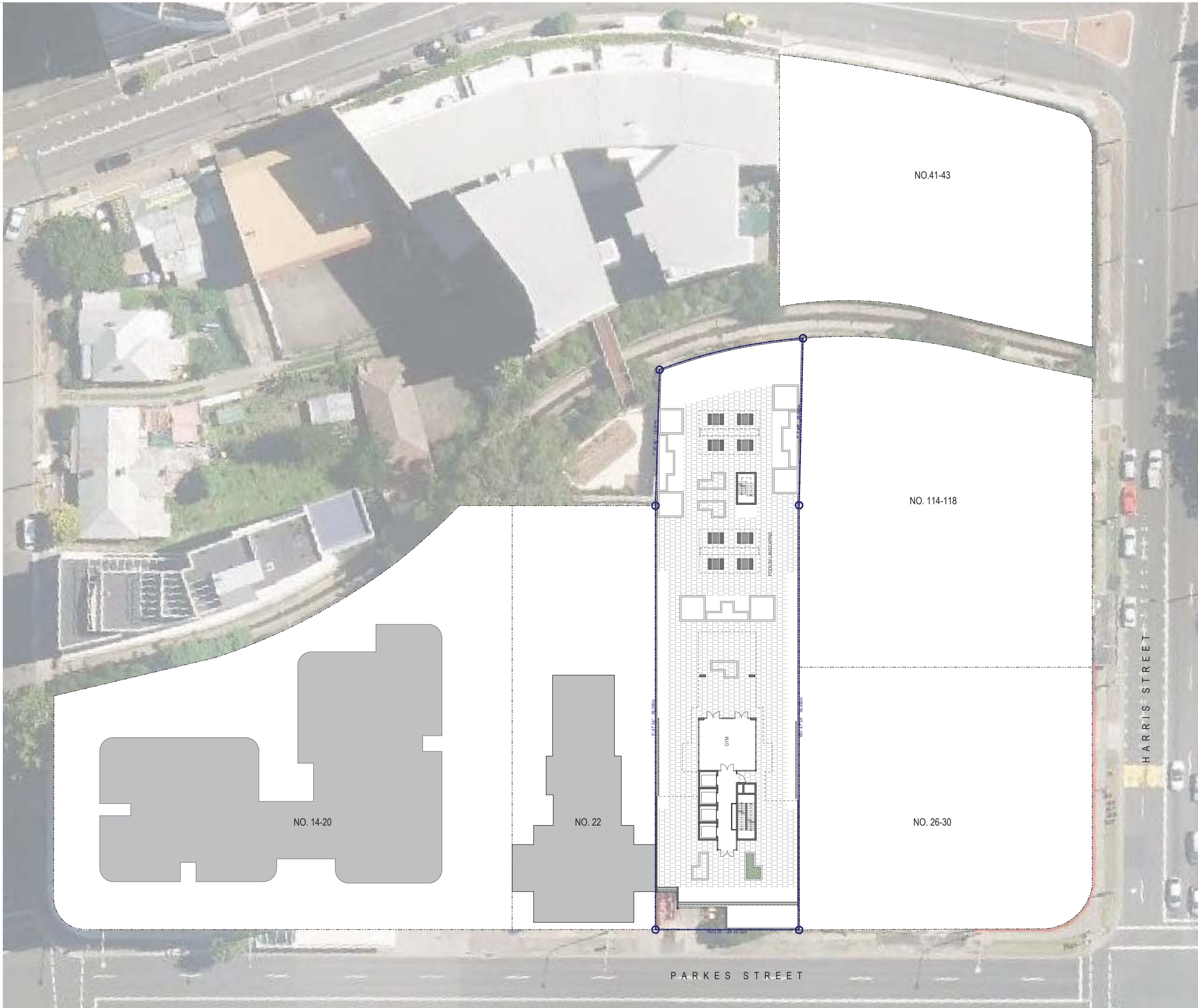
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NORTH:



SHEET TITLE:
 L2&3 - Typical Commercial

DESIGNED: AHM **DRAWN:** AH **COMMENCED:** February 2016 **SCALE:** 1:500 **PRINT:** A3 SHEET

JOB No.: 08486 **DRAWING No.:** SK-8 06 **ISSUE:** 8



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PROJECT STATUS:
 Option SK-8

PROJECT NAME:
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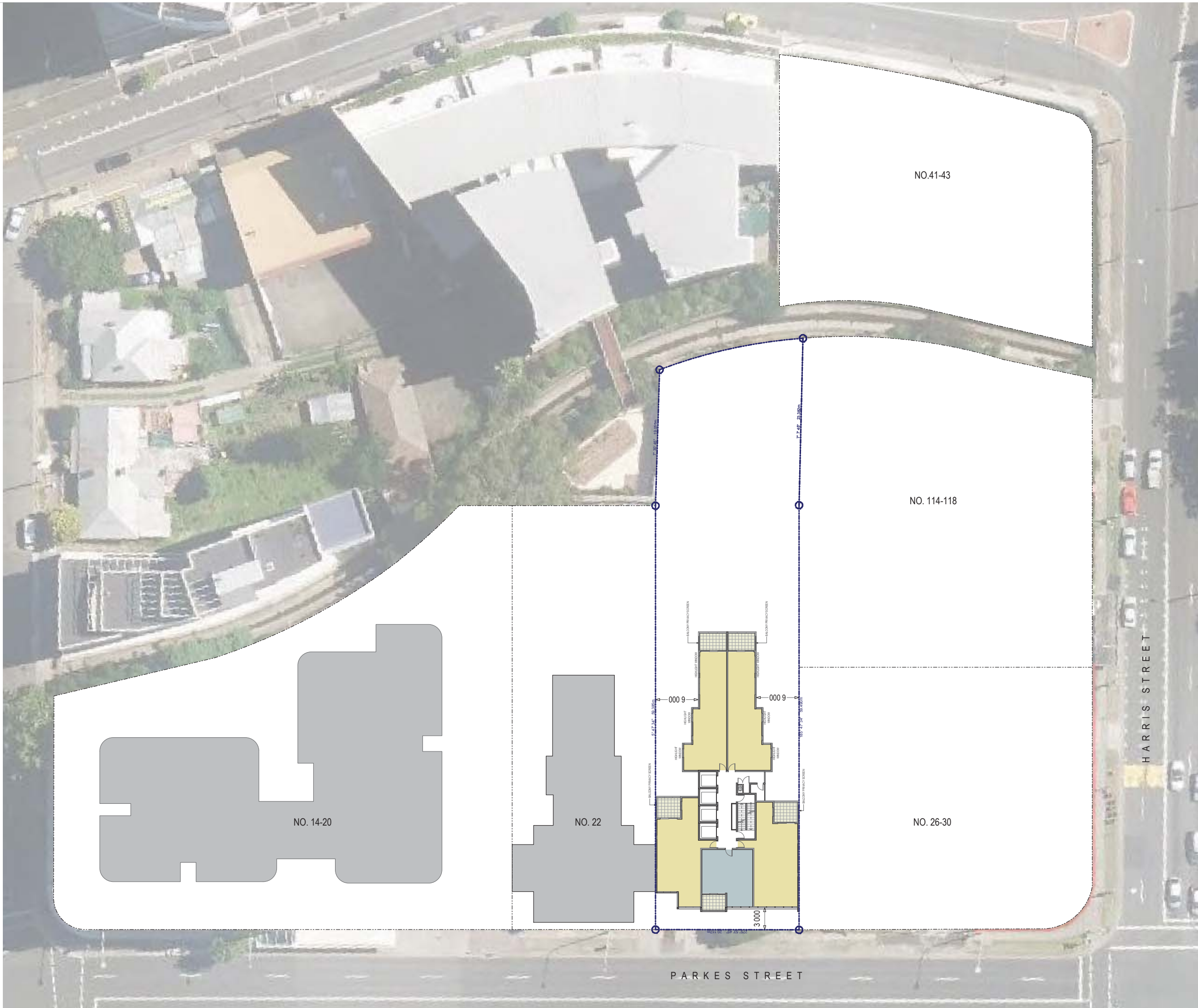
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NORTH:



SHEET TITLE:
 L4 - Podium COS

DESIGNED: AHM **DRAWN:** AH **COMMENCED:** February 2016 **SCALE:** 1:500 **PRINT:** A3 SHEET

JOB No.: 08486 **DRAWING No.:** SK-8 07 **ISSUE:** 8



- 1 BEDROOM UNIT
- 2 BEDROOM UNIT
- 3 BEDROOM UNIT

| ISSUE | AMENDMENT | DATE | DRAWN | CHECKED |
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 Option SK-8

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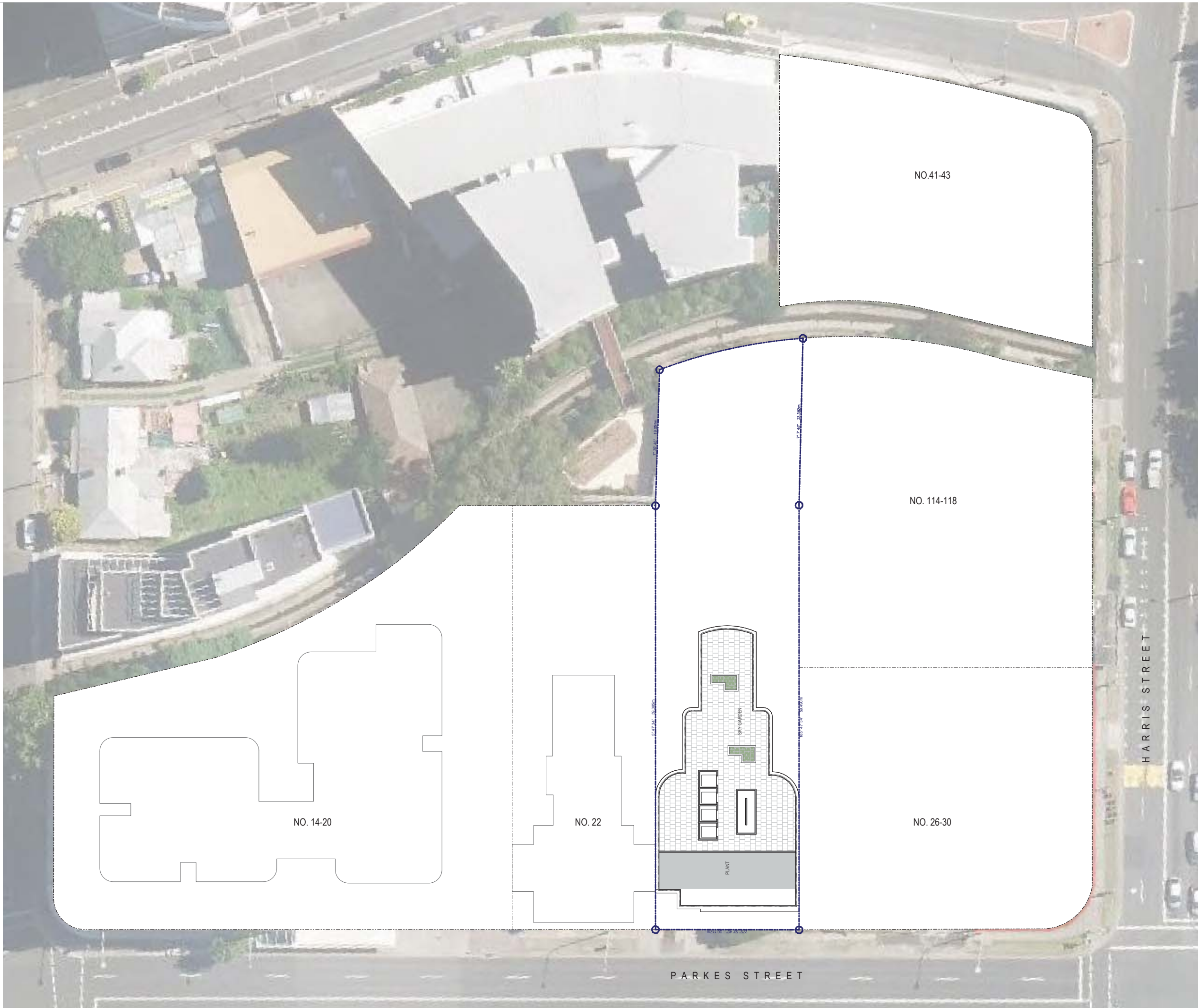
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NORTH:



SHEET TITLE:
 L5-18 + L20-41 + L43-55 -
 Typical Residential

DESIGNED: AHM **DRAWN:** AH **COMMENTED:** February 2016 **SCALE:** 1:500 **PRINT:** A3 SHEET

08486 **SK-8 08** **8**
 JOB No. DRAWING No. ISSUE



| ISSUE | AMENDMENT | DATE | DRAWN | CHECKED |
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GENERAL NOTES:
 Figured dimensions shall be taken in preference to scaling.
 Drawing to be read in conjunction with information on first page.
 Check all dimensions and levels on site before commencing work or ordering materials.
 All existing ground lines & trees location are approximate, therefore to be verified on-site by the builder.
 Any discrepancies to be verified back to Zhin Architects before proceeding.
 All workmanship and materials shall comply with all relevant codes, ordinances, Australian Standards and manufacturer's instructions.
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 28 495 869 790 / abn

PROJECT STATUS:
 Option SK-8

PROJECT NAME:
 Concept Design
 24 Parkes Street
 Parramatta NSW 2150

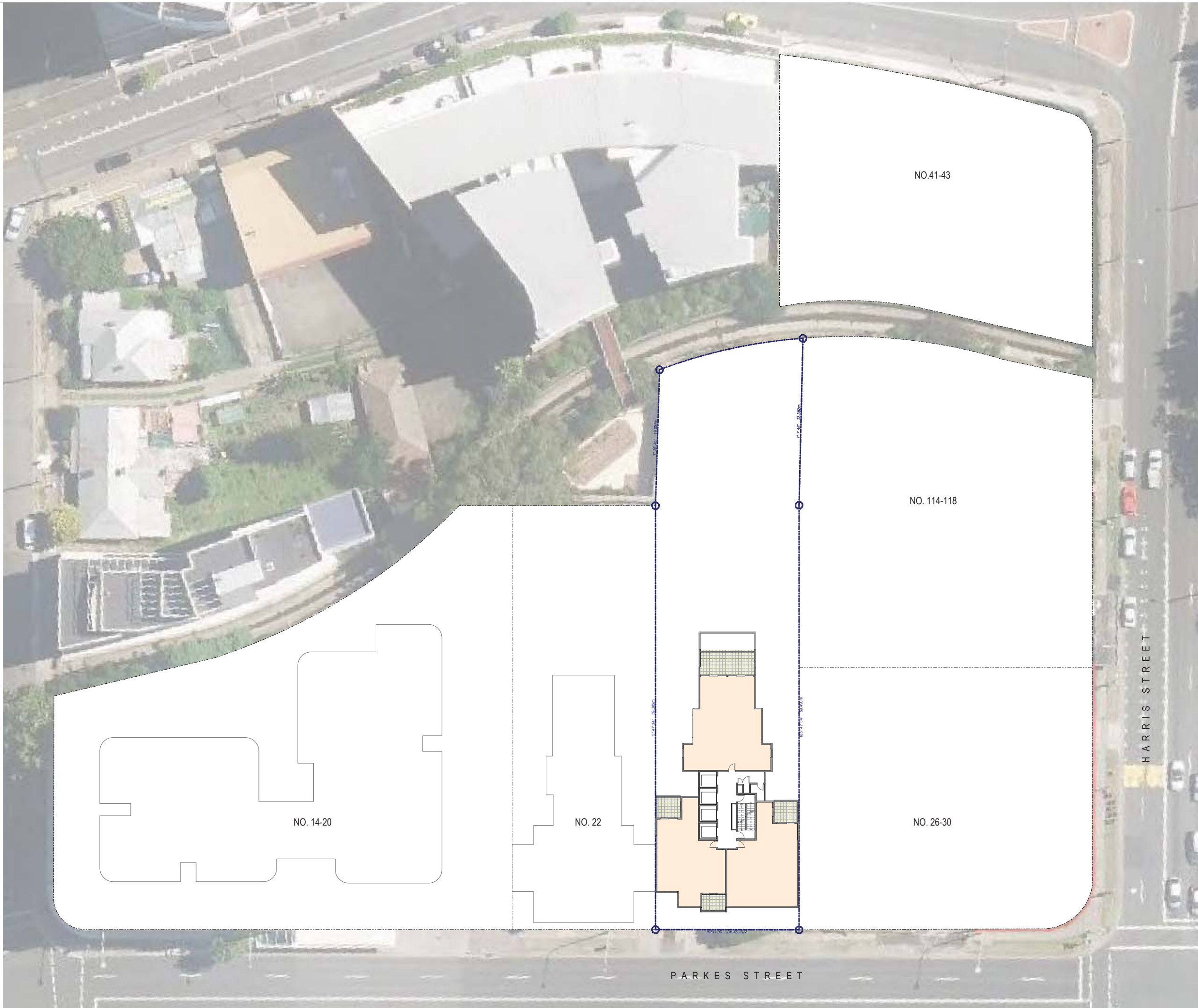
L.G.A.: Parramatta City Council
NORTH:



SHEET TITLE:
 L19+42 - Typical Service & COS

DESIGNED: AHM **DRAWN:** AH **COMMENCED:** February 2016 **SCALE:** 1:500 **PRINT:** A3 SHEET

08486 **SK-8 09** **8**
 JOB No. DRAWING No. ISSUE



- 1 BEDROOM UNIT
- 2 BEDROOM UNIT
- 3 BEDROOM UNIT

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 Check all dimensions and levels on site before commencing work or ordering materials.
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PROJECT STATUS:
 Option SK-8

PROJECT NAME:
 Concept Design
 24 Parkes Street
 Parramatta NSW 2150

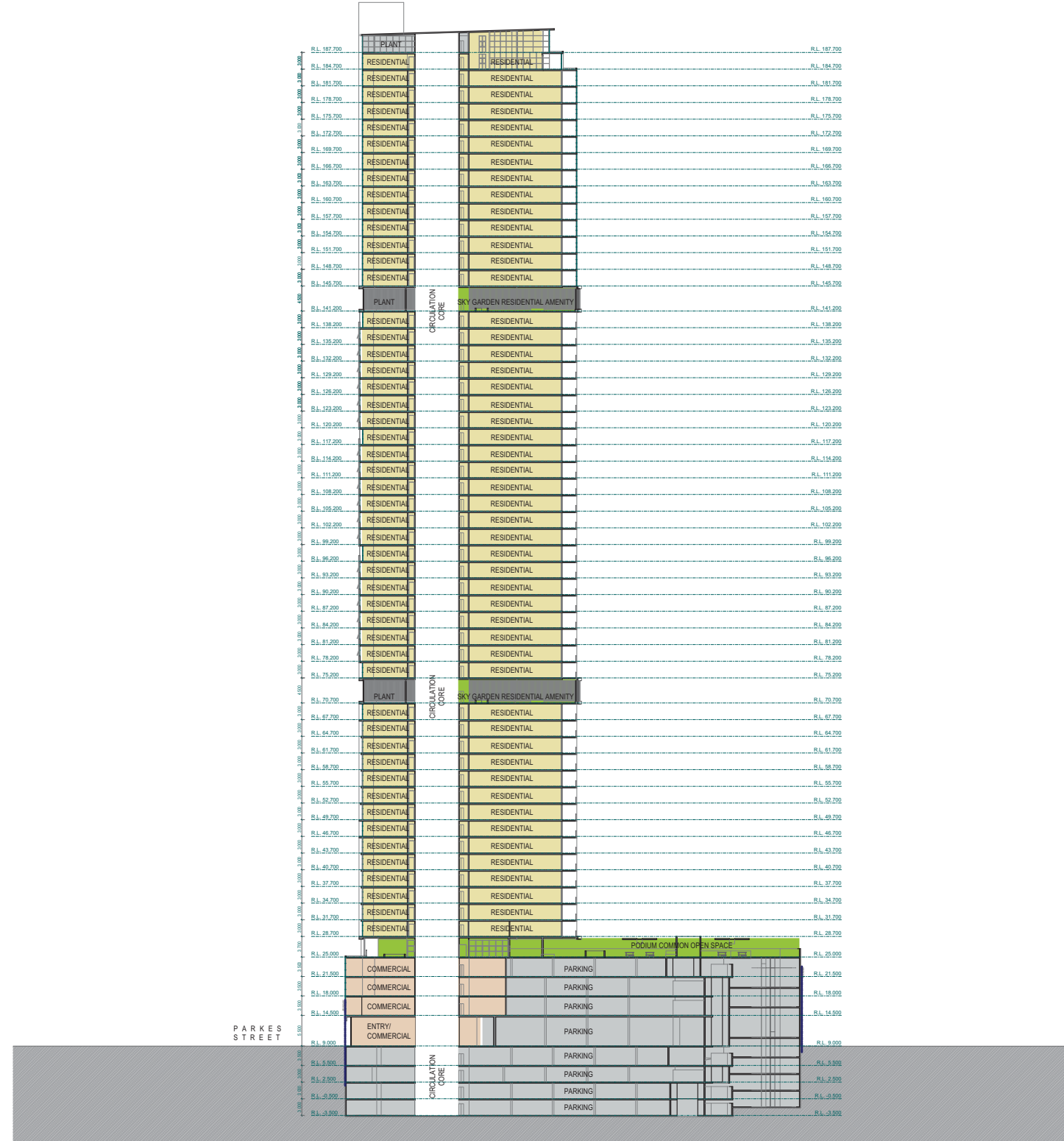
L.G.A.: Parramatta City Council
NORTH:



SHEET TITLE:
 L56 - Residential Penthouse

DESIGNED: AHM **DRAWN:** AH **COMMENCED:** February 2016 **SCALE:** **PRINT:** A3 SHEET

08486 **SK-8 10** **8**
JOB No. DRAWING No. ISSUE



LONG SECTION (east)
Scale @A1 - 1:500

| ISSUE | AMENDMENT | DATE | DRAWN | CHECKED |
|-------|-----------|------|-------|---------|
| | | | | |
| | | | | |
| | | | | |
| | | | | |

PRINT DATE: Tuesday, 10 May 2016 2:38 pm

GENERAL NOTES:
Figured dimensions shall be taken in preference to scaling. Drawing to be read in conjunction with information on first page. Check all dimensions and levels on site before commencing work or ordering materials. All existing ground lines & trees location are approximate, therefore to be verified on-site by the builder. Any discrepancies to be verified back to Zhin Architects before proceeding. All workmanship and materials shall comply with all relevant codes, ordinances, Australian Standards and manufacturer's instructions. Unless noted 'Issued for Construction', drawing not to be used for construction.
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SHEET TITLE:
Section
DESIGNED: AHM DRAWN: AH COMMENCED: February 2016 SCALE: AS NOTED PRINT: A3 SHEET
L.G.A.: Parramatta City Council



NORTH:

PROJECT STATUS:
Option SK-8

PRELIMINARY

PROJECT NAME:
Concept Design
24 Parkes Street
Parramatta NSW 2150
08486 SK-8 13
JOB No. DRAWING No.



Appendix C

SIDRA Intersection Modelling Outputs



Appendix C-1

Existing

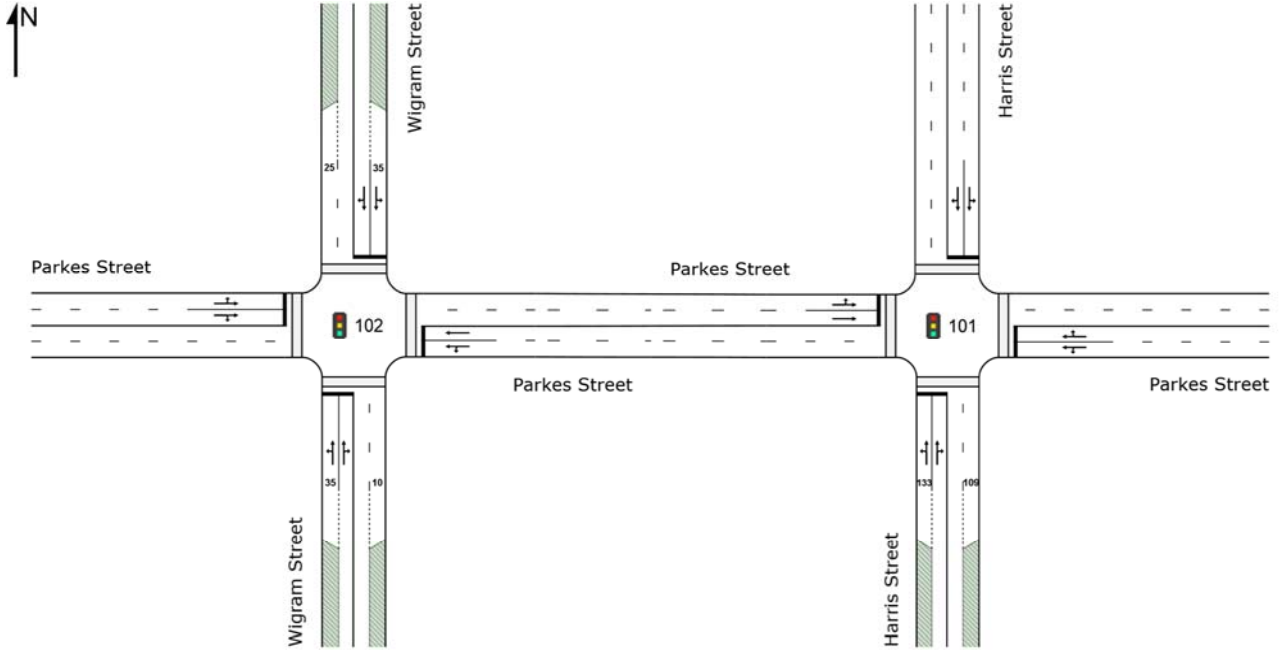
NETWORK LAYOUT

📍 Network: N101 [Parkes Street Network EX AM]

Parkes Street, Harris Street and Wigram Streets.

7:45-8:45

Network Category: Existing AM



SITES IN NETWORK

| Site ID | CCG ID | Site Name |
|---------|--------|---------------------------|
| 🚦 101 | NA | Harris St Parkes St EX AM |
| 🚦 102 | NA | Wigram St Parkes St EX AM |

SIDRA INTERSECTION 8.0 | Copyright © 2000-2018 Akcelik and Associates Pty Ltd | sidrasolutions.com

Organisation: TRAFFIX PTY LTD | Created: Wednesday, 6 June 2018 12:24:51 PM

Project: T:\Synergy\Projects\18\18.217\Modelling\18.217m01v01 TRAFFIX Parkes Street Network.sip8

MOVEMENT SUMMARY

 Site: 101 [Harris St Parkes St EX AM]

 Network: N101 [Parkes Street Network EX AM]

Harris Street and Parkes Street
7.45-8.45

Site Category: Existing AM

Signals - Fixed Time Coordinated Cycle Time = 100 seconds (Network Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------|------|---------------|------|-----------|---------------|------------------|---------------------|------------|--------------|---------------------|------------------|---------------|
| Mov ID | Turn | Demand Flows | | Arrival Flows | | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue | | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed |
| | | Total veh/h | HV % | Total veh/h | HV % | v/c | sec | | Vehicles veh | Distance m | | | | km/h |
| South: Harris Street | | | | | | | | | | | | | | |
| 1 | L2 | 48 | 10.9 | 48 | 10.9 | 0.927 | 68.1 | LOS E | 9.4 | 67.3 | 1.00 | 1.15 | 1.51 | 8.7 |
| 2 | T1 | 437 | 0.7 | 437 | 0.7 | 0.927 | 64.4 | LOS E | 9.8 | 68.9 | 1.00 | 1.15 | 1.50 | 17.3 |
| 3 | R2 | 34 | 3.1 | 34 | 3.1 | 0.927 | 67.6 | LOS E | 9.8 | 68.9 | 1.00 | 1.15 | 1.49 | 18.1 |
| Approach | | 519 | 1.8 | 519 | 1.8 | 0.927 | 65.0 | LOS E | 9.8 | 68.9 | 1.00 | 1.15 | 1.50 | 16.8 |
| East: Parkes Street | | | | | | | | | | | | | | |
| 4 | L2 | 44 | 4.8 | 44 | 4.8 | 0.686 | 29.7 | LOS C | 13.4 | 98.2 | 0.86 | 0.78 | 0.86 | 31.2 |
| 5 | T1 | 514 | 5.9 | 514 | 5.9 | 0.686 | 24.1 | LOS B | 13.4 | 98.2 | 0.86 | 0.78 | 0.86 | 26.4 |
| 6 | R2 | 309 | 3.4 | 309 | 3.4 | 1.004 | 97.3 | LOS F | 14.1 | 101.9 | 1.00 | 1.17 | 1.81 | 16.0 |
| Approach | | 867 | 5.0 | 867 | 5.0 | 1.004 | 50.5 | LOS D | 14.1 | 101.9 | 0.91 | 0.92 | 1.20 | 19.9 |
| North: Harris Street | | | | | | | | | | | | | | |
| 7 | L2 | 62 | 5.1 | 62 | 5.1 | 0.585 | 40.2 | LOS C | 6.8 | 48.7 | 0.94 | 0.80 | 0.94 | 26.3 |
| 8 | T1 | 272 | 0.8 | 272 | 0.8 | 0.975 | 48.1 | LOS D | 17.5 | 124.8 | 0.96 | 0.92 | 1.13 | 20.3 |
| 9 | R2 | 335 | 2.2 | 335 | 2.2 | 0.975 | 79.3 | LOS F | 17.5 | 124.8 | 1.00 | 1.23 | 1.60 | 11.3 |
| Approach | | 668 | 1.9 | 668 | 1.9 | 0.975 | 63.0 | LOS E | 17.5 | 124.8 | 0.98 | 1.06 | 1.35 | 15.9 |
| West: Parkes Street | | | | | | | | | | | | | | |
| 10 | L2 | 275 | 3.1 | 275 | 3.1 | 0.925 | 31.3 | LOS C | 10.5 | 75.9 | 0.96 | 0.98 | 1.29 | 25.5 |
| 11 | T1 | 486 | 6.9 | 486 | 6.9 | 0.925 | 56.8 | LOS E | 13.5 | 99.9 | 0.99 | 1.09 | 1.39 | 19.3 |
| Approach | | 761 | 5.5 | 761 | 5.5 | 0.925 | 47.6 | LOS D | 13.5 | 99.9 | 0.98 | 1.05 | 1.36 | 21.2 |
| All Vehicles | | 2816 | 3.8 | 2816 | 3.8 | 1.004 | 55.4 | LOS D | 17.5 | 124.8 | 0.96 | 1.03 | 1.33 | 18.5 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------|---------------|------------------|-----------------------|--------------|---------------------|------|--|
| Mov ID | Description | Demand Flow | Average Delay | Level of Service | Average Back of Queue | Prop. Queued | Effective Stop Rate | | |
| | | ped/h | sec | | Pedestrian ped | Distance m | | | |
| P1 | South Full Crossing | 53 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | |
| P2 | East Full Crossing | 53 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | |
| P3 | North Full Crossing | 53 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | |
| P4 | West Full Crossing | 53 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | |
| All Pedestrians | | 211 | 44.3 | LOS E | | | 0.94 | 0.94 | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

MOVEMENT SUMMARY

 Site: 102 [Wigram St Parkes St EX AM]

 Network: N101 [Parkes Street Network EX AM]

Wigram Street and Harris Street
7.45-8.45

Site Category: Existing AM

Signals - Fixed Time Coordinated Cycle Time = 100 seconds (Network Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------|------|---------------|------|-----------|---------------|------------------|---------------------|------------|--------------|---------------------|------------------|---------------|
| Mov ID | Turn | Demand Flows | | Arrival Flows | | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue | | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed |
| | | Total veh/h | HV % | Total veh/h | HV % | | | | Vehicles | Distance m | | | | |
| South: Wigram Street | | | | | | | | | | | | | | |
| 1 | L2 | 54 | 11.8 | 54 | 11.8 | 0.147 | 36.4 | LOS C | 1.5 | 11.5 | 0.82 | 0.72 | 0.82 | 26.6 |
| 2 | T1 | 139 | 0.8 | 139 | 0.8 | 0.635 | 44.7 | LOS D | 4.4 | 31.2 | 0.98 | 0.81 | 1.01 | 21.9 |
| 3 | R2 | 22 | 0.0 | 22 | 0.0 | 0.635 | 50.3 | LOS D | 4.4 | 31.2 | 0.99 | 0.82 | 1.03 | 19.0 |
| Approach | | 215 | 3.4 | 215 | 3.4 | 0.635 | 43.2 | LOS D | 4.4 | 31.2 | 0.94 | 0.79 | 0.97 | 22.8 |
| East: Parkes Street | | | | | | | | | | | | | | |
| 4 | L2 | 35 | 3.0 | 35 | 3.0 | 0.653 | 22.8 | LOS B | 8.6 | 62.0 | 0.68 | 0.61 | 0.68 | 34.9 |
| 5 | T1 | 853 | 4.0 | 853 | 4.0 | 0.653 | 23.2 | LOS B | 11.0 | 79.9 | 0.79 | 0.70 | 0.79 | 27.3 |
| Approach | | 887 | 3.9 | 887 | 3.9 | 0.653 | 23.2 | LOS B | 11.0 | 79.9 | 0.79 | 0.70 | 0.79 | 27.7 |
| North: Wigram Street | | | | | | | | | | | | | | |
| 7 | L2 | 38 | 8.3 | 38 | 8.3 | 0.164 | 45.3 | LOS D | 1.0 | 7.5 | 0.90 | 0.72 | 0.90 | 7.7 |
| 8 | T1 | 65 | 3.2 | 65 | 3.2 | 0.452 | 47.1 | LOS D | 2.6 | 18.7 | 0.98 | 0.77 | 0.98 | 21.2 |
| 9 | R2 | 23 | 4.5 | 23 | 4.5 | 0.452 | 51.7 | LOS D | 2.6 | 18.7 | 0.98 | 0.77 | 0.98 | 15.1 |
| Approach | | 126 | 5.0 | 126 | 5.0 | 0.452 | 47.4 | LOS D | 2.6 | 18.7 | 0.96 | 0.75 | 0.96 | 17.2 |
| West: Parkes Street | | | | | | | | | | | | | | |
| 10 | L2 | 99 | 1.1 | 99 | 1.1 | 0.646 | 28.5 | LOS B | 12.4 | 90.5 | 0.83 | 0.76 | 0.83 | 24.1 |
| 11 | T1 | 703 | 5.5 | 703 | 5.5 | 0.646 | 24.3 | LOS B | 12.4 | 90.5 | 0.84 | 0.76 | 0.84 | 18.6 |
| 12 | R2 | 97 | 3.3 | 97 | 3.3 | 0.646 | 32.3 | LOS C | 8.5 | 62.1 | 0.84 | 0.75 | 0.84 | 30.3 |
| Approach | | 899 | 4.8 | 899 | 4.8 | 0.646 | 25.6 | LOS B | 12.4 | 90.5 | 0.84 | 0.76 | 0.84 | 21.3 |
| All Vehicles | | 2127 | 4.3 | 2127 | 4.3 | 0.653 | 27.7 | LOS B | 12.4 | 90.5 | 0.83 | 0.73 | 0.84 | 23.6 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|--------------------------------------|------------|--------------|---------------------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian ped | Distance m | Prop. Queued | Effective Stop Rate | |
| P1 | South Full Crossing | 53 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | |
| P2 | East Full Crossing | 53 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | |
| P3 | North Full Crossing | 53 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | |
| P4 | West Full Crossing | 53 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | |
| All Pedestrians | | 211 | 44.3 | LOS E | | | 0.94 | 0.94 | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

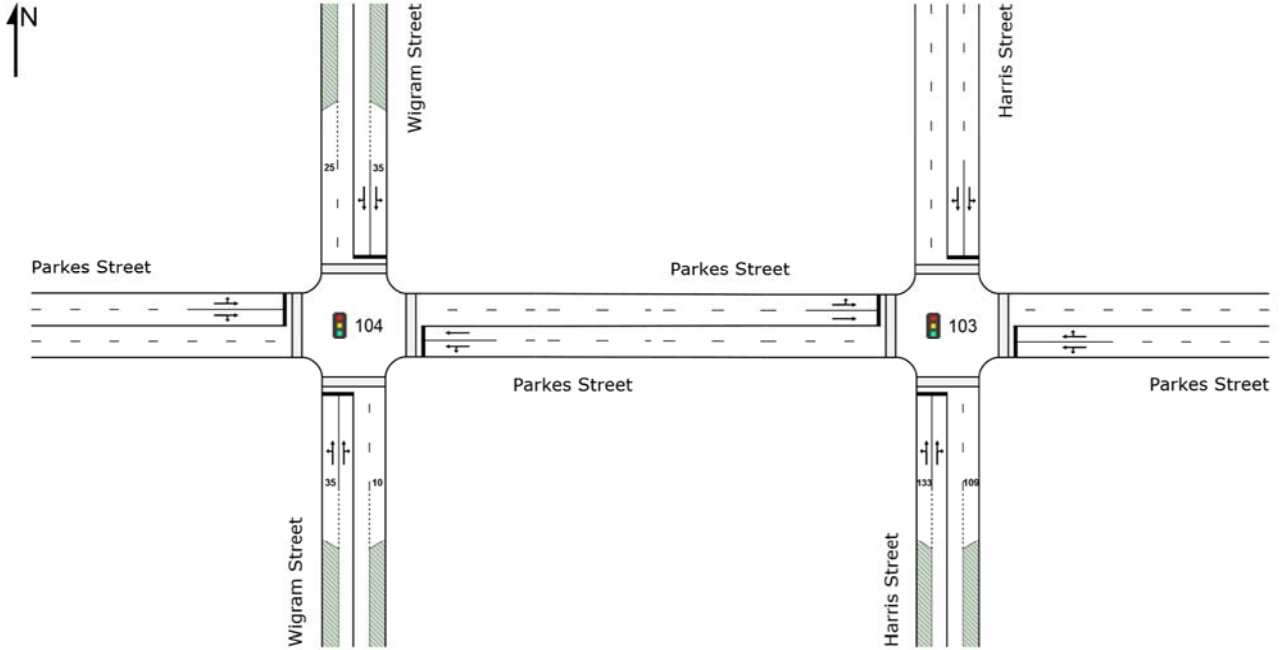
Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

NETWORK LAYOUT

📍 Network: N102 [Parkes Street Network EX PM]

Parkes Street, Harris Street and Wigram Streets.
 4:30-5:30
 Network Category: Existing PM



| SITES IN NETWORK | | |
|------------------|--------|---------------------------|
| Site ID | CCG ID | Site Name |
| 🚦 103 | NA | Harris St Parkes St EX PM |
| 🚦 104 | NA | Wigram St Parkes St EX PM |

MOVEMENT SUMMARY

 Site: 103 [Harris St Parkes St EX PM]

 Network: N102 [Parkes Street Network EX PM]

Harris Street and Parkes Street
4:30-5:30

Site Category: Existing PM

Signals - Fixed Time Coordinated Cycle Time = 100 seconds (Network Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------|------|---------------|------|-----------|---------------|------------------|---------------------|------------|--------------|---------------------|------------------|---------------|
| Mov ID | Turn | Demand Flows | | Arrival Flows | | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue | | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed |
| | | Total veh/h | HV % | Total veh/h | HV % | | | | Vehicles | Distance m | | | | |
| South: Harris Street | | | | | | | | | | | | | | |
| 1 | L2 | 51 | 0.0 | 51 | 0.0 | 0.987 | 88.2 | LOS F | 8.8 | 62.1 | 1.00 | 1.27 | 1.81 | 7.0 |
| 2 | T1 | 354 | 0.6 | 354 | 0.6 | 0.987 | 83.3 | LOS F | 9.6 | 67.3 | 1.00 | 1.28 | 1.79 | 16.1 |
| 3 | R2 | 31 | 0.0 | 31 | 0.0 | 0.987 | 87.5 | LOS F | 9.6 | 67.3 | 1.00 | 1.28 | 1.78 | 15.2 |
| Approach | | 435 | 0.5 | 435 | 0.5 | 0.987 | 84.1 | LOS F | 9.6 | 67.3 | 1.00 | 1.28 | 1.79 | 15.1 |
| East: Parkes Street | | | | | | | | | | | | | | |
| 4 | L2 | 32 | 0.0 | 32 | 0.0 | 0.716 | 31.3 | LOS C | 12.5 | 89.3 | 0.87 | 0.77 | 0.87 | 30.5 |
| 5 | T1 | 484 | 2.6 | 484 | 2.6 | 0.716 | 25.7 | LOS B | 12.5 | 89.3 | 0.87 | 0.77 | 0.87 | 25.5 |
| 6 | R2 | 228 | 0.5 | 228 | 0.5 | 1.028 | 110.4 | LOS F | 11.0 | 77.5 | 1.00 | 1.22 | 1.99 | 15.3 |
| Approach | | 744 | 1.8 | 744 | 1.8 | 1.028 | 52.0 | LOS D | 12.5 | 89.3 | 0.91 | 0.91 | 1.21 | 19.6 |
| North: Harris Street | | | | | | | | | | | | | | |
| 7 | L2 | 64 | 3.3 | 64 | 3.3 | 0.634 | 38.5 | LOS C | 9.1 | 64.5 | 0.92 | 0.81 | 0.92 | 30.9 |
| 8 | T1 | 376 | 0.8 | 376 | 0.8 | 1.056 | 55.7 | LOS D | 28.3 | 200.6 | 0.94 | 0.95 | 1.18 | 20.6 |
| 9 | R2 | 417 | 1.5 | 417 | 1.5 | 1.056 | 128.2 | LOS F | 28.3 | 200.6 | 1.00 | 1.36 | 2.00 | 8.3 |
| Approach | | 857 | 1.4 | 857 | 1.4 | 1.056 | 89.7 | LOS F | 28.3 | 200.6 | 0.97 | 1.14 | 1.56 | 13.5 |
| West: Parkes Street | | | | | | | | | | | | | | |
| 10 | L2 | 287 | 0.7 | 287 | 0.7 | 0.968 | 51.0 | LOS D | 15.4 | 108.9 | 1.00 | 1.11 | 1.54 | 21.8 |
| 11 | T1 | 588 | 2.3 | 588 | 2.3 | 0.968 | 70.9 | LOS F | 17.6 | 125.8 | 1.00 | 1.20 | 1.56 | 16.5 |
| Approach | | 876 | 1.8 | 876 | 1.8 | 0.968 | 64.4 | LOS E | 17.6 | 125.8 | 1.00 | 1.17 | 1.55 | 18.0 |
| All Vehicles | | 2912 | 1.5 | 2912 | 1.5 | 1.056 | 71.6 | LOS F | 28.3 | 200.6 | 0.97 | 1.11 | 1.50 | 16.3 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|--------------------------------------|------------|--------------|---------------------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian ped | Distance m | Prop. Queued | Effective Stop Rate | |
| P1 | South Full Crossing | 53 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | |
| P2 | East Full Crossing | 53 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | |
| P3 | North Full Crossing | 53 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | |
| P4 | West Full Crossing | 53 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | |
| All Pedestrians | | 211 | 44.3 | LOS E | | | 0.94 | 0.94 | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

MOVEMENT SUMMARY

Site: 104 [Wigram St Parkes St EX PM]

Network: N102 [Parkes Street Network EX PM]

Wigram Street and Harris Street
4:30-5:30

Site Category: Existing PM

Signals - Fixed Time Coordinated Cycle Time = 100 seconds (Network Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------|------|--------------------|------|-----------|---------------|------------------|---------------------|------------|--------------|---------------------|------------------|---------------|
| Mov ID | Turn | Demand Flows | | Arrival Flows | | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue | | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed |
| | | Total veh/h | HV % | Total veh/h | HV % | v/c | sec | | Vehicles veh | Distance m | | | | km/h |
| South: Wigram Street | | | | | | | | | | | | | | |
| 1 | L2 | 85 | 6.2 | 85 | 6.2 | 0.169 | 29.4 | LOS C | 2.1 | 15.5 | 0.74 | 0.71 | 0.74 | 29.4 |
| 2 | T1 | 84 | 1.3 | 84 | 1.3 | 0.732 | 46.3 | LOS D | 3.2 | 22.3 | 0.95 | 0.85 | 1.12 | 21.3 |
| 3 | R2 | 33 | 0.0 | 33 | 0.0 | 0.732 | 55.9 | LOS D | 3.2 | 22.3 | 1.00 | 0.88 | 1.21 | 17.5 |
| Approach | | 202 | 3.1 | 202 | 3.1 | 0.732 | 40.7 | LOS C | 3.2 | 22.3 | 0.87 | 0.79 | 0.97 | 23.7 |
| East: Parkes Street | | | | | | | | | | | | | | |
| 4 | L2 | 40 | 5.3 | 39 | 5.3 | 0.829 | 29.7 | LOS C | 11.9 | 85.2 | 0.87 | 0.81 | 0.94 | 30.9 |
| 5 | T1 | 887 | 2.6 | 875 | 2.6 | 0.829 | 31.7 | LOS C | 13.4 | 95.6 | 0.93 | 0.87 | 1.00 | 22.8 |
| Approach | | 927 | 2.7 | 914 ^{N1} | 2.7 | 0.829 | 31.6 | LOS C | 13.4 | 95.6 | 0.93 | 0.87 | 1.00 | 23.2 |
| North: Wigram Street | | | | | | | | | | | | | | |
| 7 | L2 | 49 | 0.0 | 49 | 0.0 | 0.197 | 37.6 | LOS C | 1.3 | 9.3 | 0.83 | 0.71 | 0.83 | 9.1 |
| 8 | T1 | 145 | 1.4 | 145 | 1.4 | 0.987 | 82.5 | LOS F | 8.2 | 59.3 | 0.99 | 1.25 | 1.79 | 14.9 |
| 9 | R2 | 54 | 7.8 | 54 | 7.8 | 0.987 | 89.2 | LOS F | 8.2 | 59.3 | 1.00 | 1.27 | 1.83 | 9.8 |
| Approach | | 248 | 2.5 | 248 | 2.5 | 0.987 | 75.0 | LOS F | 8.2 | 59.3 | 0.96 | 1.15 | 1.60 | 13.2 |
| West: Parkes Street | | | | | | | | | | | | | | |
| 10 | L2 | 80 | 1.3 | 80 | 1.3 | 0.981 | 78.6 | LOS F | 22.5 | 159.9 | 0.90 | 1.24 | 1.50 | 11.2 |
| 11 | T1 | 797 | 2.0 | 797 | 2.0 | 0.981 | 74.2 | LOS F | 22.5 | 159.9 | 0.89 | 1.23 | 1.51 | 8.0 |
| 12 | R2 | 92 | 5.7 | 92 | 5.7 | 0.981 | 81.3 | LOS F | 17.6 | 126.3 | 0.87 | 1.21 | 1.53 | 17.6 |
| Approach | | 968 | 2.3 | 968 | 2.3 | 0.981 | 75.2 | LOS F | 22.5 | 159.9 | 0.89 | 1.23 | 1.51 | 9.4 |
| All Vehicles | | 2346 | 2.6 | 2333 ^{N1} | 2.6 | 0.987 | 55.1 | LOS D | 22.5 | 159.9 | 0.91 | 1.04 | 1.27 | 15.0 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

^{N1} Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|--------------------------------------|------------|--------------|---------------------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian ped | Distance m | Prop. Queued | Effective Stop Rate | |
| P1 | South Full Crossing | 53 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | |
| P2 | East Full Crossing | 53 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | |
| P3 | North Full Crossing | 53 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | |
| P4 | West Full Crossing | 53 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | |
| All Pedestrians | | 211 | 44.3 | LOS E | | | 0.94 | 0.94 | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.



Appendix C-2

Existing + Development (No Improvements)

MOVEMENT SUMMARY

 Site: 201 [Harris St Parkes St EX + FU AM]

 Network: N201 [Parkes Street Network EX + FU AM]

Harris Street and Parkes Street
7.45-8.45

Site Category: Future AM

Signals - Fixed Time Coordinated Cycle Time = 100 seconds (Network Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------|------|---------------|------|-----------|---------------|------------------|---------------------|------------|--------------|---------------------|------------------|---------------|
| Mov ID | Turn | Demand Flows | | Arrival Flows | | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue | | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed |
| | | Total veh/h | HV % | Total veh/h | HV % | | | | Vehicles | Distance m | | | | |
| South: Harris Street | | | | | | | | | | | | | | |
| 1 | L2 | 49 | 10.6 | 49 | 10.6 | 1.093 | 154.8 | LOS F | 15.0 | 107.6 | 1.00 | 1.61 | 2.33 | 4.1 |
| 2 | T1 | 440 | 0.7 | 440 | 0.7 | 1.093 | 151.0 | LOS F | 16.0 | 113.2 | 1.00 | 1.62 | 2.32 | 9.4 |
| 3 | R2 | 34 | 3.1 | 34 | 3.1 | 1.093 | 154.0 | LOS F | 16.0 | 113.2 | 1.00 | 1.62 | 2.31 | 9.4 |
| Approach | | 523 | 1.8 | 523 | 1.8 | 1.093 | 151.6 | LOS F | 16.0 | 113.2 | 1.00 | 1.62 | 2.32 | 9.0 |
| East: Parkes Street | | | | | | | | | | | | | | |
| 4 | L2 | 44 | 4.8 | 44 | 4.8 | 0.686 | 29.7 | LOS C | 13.4 | 98.2 | 0.86 | 0.78 | 0.86 | 31.2 |
| 5 | T1 | 520 | 5.9 | 520 | 5.9 | 1.074 | 25.5 | LOS B | 19.0 | 136.4 | 0.86 | 0.78 | 0.88 | 26.1 |
| 6 | R2 | 325 | 3.2 | 325 | 3.2 | 1.074 | 141.9 | LOS F | 19.0 | 136.4 | 1.00 | 1.34 | 2.18 | 12.1 |
| Approach | | 889 | 4.9 | 889 | 4.9 | 1.074 | 68.3 | LOS E | 19.0 | 136.4 | 0.91 | 0.98 | 1.35 | 16.2 |
| North: Harris Street | | | | | | | | | | | | | | |
| 7 | L2 | 95 | 3.3 | 95 | 3.3 | 0.648 | 40.1 | LOS C | 8.2 | 58.2 | 0.95 | 0.81 | 0.95 | 26.3 |
| 8 | T1 | 279 | 0.8 | 279 | 0.8 | 1.080 | 63.6 | LOS E | 26.5 | 188.1 | 0.96 | 1.01 | 1.26 | 17.3 |
| 9 | R2 | 373 | 2.0 | 373 | 2.0 | 1.080 | 144.4 | LOS F | 26.5 | 188.1 | 1.00 | 1.57 | 2.16 | 7.0 |
| Approach | | 746 | 1.7 | 746 | 1.7 | 1.080 | 101.0 | LOS F | 26.5 | 188.1 | 0.98 | 1.27 | 1.67 | 11.5 |
| West: Parkes Street | | | | | | | | | | | | | | |
| 10 | L2 | 299 | 2.8 | 299 | 2.8 | 0.979 | 46.4 | LOS D | 13.1 | 94.9 | 1.00 | 1.06 | 1.45 | 20.8 |
| 11 | T1 | 501 | 6.7 | 501 | 6.7 | 0.979 | 71.5 | LOS F | 15.6 | 115.3 | 1.00 | 1.17 | 1.52 | 16.5 |
| Approach | | 800 | 5.3 | 800 | 5.3 | 0.979 | 62.1 | LOS E | 15.6 | 115.3 | 1.00 | 1.13 | 1.49 | 17.9 |
| All Vehicles | | 2959 | 3.6 | 2959 | 3.6 | 1.093 | 89.6 | LOS F | 26.5 | 188.1 | 0.97 | 1.21 | 1.64 | 13.2 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|--------------------------------------|------------|--------------|---------------------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian ped | Distance m | Prop. Queued | Effective Stop Rate | |
| P1 | South Full Crossing | 53 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | |
| P2 | East Full Crossing | 53 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | |
| P3 | North Full Crossing | 53 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | |
| P4 | West Full Crossing | 53 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | |
| All Pedestrians | | 211 | 44.3 | LOS E | | | 0.94 | 0.94 | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

MOVEMENT SUMMARY

 Site: 203 [Harris St Parkes St EX + FU PM]

 Network: N202 [Parkes Street Network EX + FU PM]

Harris Street and Parkes Street
4:30-5:30

Site Category: Future PM

Signals - Fixed Time Coordinated Cycle Time = 100 seconds (Network Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------|------|--------------------|------|-----------|---------------|------------------|---------------------|------------|--------------|---------------------|------------------|---------------|
| Mov ID | Turn | Demand Flows | | Arrival Flows | | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue | | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed |
| | | Total veh/h | HV % | Total veh/h | HV % | | | | Vehicles | Distance m | | | | |
| South: Harris Street | | | | | | | | | | | | | | |
| 1 | L2 | 53 | 0.0 | 53 | 0.0 | 1.016 | 103.5 | LOS F | 9.8 | 69.0 | 1.00 | 1.35 | 1.95 | 6.0 |
| 2 | T1 | 360 | 0.6 | 360 | 0.6 | 1.016 | 98.2 | LOS F | 10.8 | 75.8 | 1.00 | 1.36 | 1.93 | 14.2 |
| 3 | R2 | 31 | 0.0 | 31 | 0.0 | 1.016 | 102.1 | LOS F | 10.8 | 75.8 | 1.00 | 1.36 | 1.92 | 13.5 |
| Approach | | 443 | 0.5 | 443 | 0.5 | 1.016 | 99.1 | LOS F | 10.8 | 75.8 | 1.00 | 1.36 | 1.93 | 13.3 |
| East: Parkes Street | | | | | | | | | | | | | | |
| 4 | L2 | 32 | 0.0 | 32 | 0.0 | 0.732 | 33.2 | LOS C | 13.2 | 94.3 | 0.91 | 0.82 | 0.92 | 29.5 |
| 5 | T1 | 494 | 2.6 | 494 | 2.6 | 1.111 | 30.3 | LOS C | 16.8 | 118.2 | 0.91 | 0.83 | 0.95 | 24.1 |
| 6 | R2 | 258 | 0.4 | 258 | 0.4 | 1.111 | 170.0 | LOS F | 16.8 | 118.2 | 1.00 | 1.43 | 2.42 | 10.9 |
| Approach | | 783 | 1.7 | 783 | 1.7 | 1.111 | 76.4 | LOS F | 16.8 | 118.2 | 0.94 | 1.03 | 1.43 | 15.0 |
| North: Harris Street | | | | | | | | | | | | | | |
| 7 | L2 | 78 | 2.7 | 78 | 2.7 | 0.660 | 38.1 | LOS C | 9.7 | 68.8 | 0.93 | 0.82 | 0.93 | 31.1 |
| 8 | T1 | 379 | 0.8 | 379 | 0.8 | 1.100 | 61.2 | LOS E | 33.1 | 234.7 | 0.94 | 0.97 | 1.22 | 19.4 |
| 9 | R2 | 436 | 1.4 | 436 | 1.4 | 1.100 | 160.9 | LOS F | 33.1 | 234.7 | 1.00 | 1.48 | 2.25 | 6.7 |
| Approach | | 893 | 1.3 | 893 | 1.3 | 1.100 | 107.9 | LOS F | 33.1 | 234.7 | 0.97 | 1.21 | 1.70 | 11.7 |
| West: Parkes Street | | | | | | | | | | | | | | |
| 10 | L2 | 320 | 0.7 | 300 | 0.7 | 1.036 | 83.8 | LOS F | 17.4 | 122.7 | 1.00 | 1.21 | 1.74 | 13.6 |
| 11 | T1 | 593 | 2.3 | 555 ^{N1} | 2.3 | 1.036 | 102.4 | LOS F | 18.9 | 135.0 | 1.00 | 1.39 | 1.87 | 12.1 |
| Approach | | 913 | 1.7 | 855 ^{N1} | 1.7 | 1.036 | 95.8 | LOS F | 18.9 | 135.0 | 1.00 | 1.33 | 1.82 | 12.6 |
| All Vehicles | | 3032 | 1.4 | 2974 ^{N1} | 1.5 | 1.111 | 94.8 | LOS F | 33.1 | 234.7 | 0.98 | 1.22 | 1.70 | 12.9 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

^{N1} Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|--------------------------------------|------------|--------------|---------------------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian ped | Distance m | Prop. Queued | Effective Stop Rate | |
| P1 | South Full Crossing | 53 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | |
| P2 | East Full Crossing | 53 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | |
| P3 | North Full Crossing | 53 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | |
| P4 | West Full Crossing | 53 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | |
| All Pedestrians | | 211 | 44.3 | LOS E | | | 0.94 | 0.94 | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

MOVEMENT SUMMARY

 Site: 202 [Wigram St Parkes St EX + FU AM]

 Network: N201 [Parkes Street Network EX + FU AM]

Wigram Street and Harris Street
7.45-8.45

Site Category: Future AM

Signals - Fixed Time Coordinated Cycle Time = 100 seconds (Network Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------|------|--------------------|------|-----------|---------------|------------------|---------------------|------------|--------------|---------------------|------------------|---------------|
| Mov ID | Turn | Demand Flows | | Arrival Flows | | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue | | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed |
| | | Total veh/h | HV % | Total veh/h | HV % | | | | Vehicles | Distance m | | | | |
| South: Wigram Street | | | | | | | | | | | | | | |
| 1 | L2 | 54 | 11.8 | 54 | 11.8 | 0.181 | 39.2 | LOS C | 1.7 | 12.5 | 0.85 | 0.72 | 0.85 | 25.7 |
| 2 | T1 | 139 | 0.8 | 139 | 0.8 | 0.781 | 50.6 | LOS D | 4.7 | 33.4 | 0.99 | 0.91 | 1.19 | 20.4 |
| 3 | R2 | 22 | 0.0 | 22 | 0.0 | 0.781 | 56.9 | LOS E | 4.7 | 33.4 | 1.00 | 0.92 | 1.23 | 17.5 |
| Approach | | 215 | 3.4 | 215 | 3.4 | 0.781 | 48.4 | LOS D | 4.7 | 33.4 | 0.95 | 0.86 | 1.11 | 21.4 |
| East: Parkes Street | | | | | | | | | | | | | | |
| 4 | L2 | 35 | 3.0 | 34 | 3.0 | 0.687 | 18.0 | LOS B | 7.4 | 53.7 | 0.58 | 0.53 | 0.58 | 38.2 |
| 5 | T1 | 900 | 3.7 | 875 | 3.8 | 0.687 | 23.8 | LOS B | 12.5 | 90.7 | 0.79 | 0.70 | 0.79 | 27.0 |
| Approach | | 935 | 3.7 | 908 ^{N1} | 3.7 | 0.687 | 23.6 | LOS B | 12.5 | 90.7 | 0.78 | 0.70 | 0.78 | 27.5 |
| North: Wigram Street | | | | | | | | | | | | | | |
| 7 | L2 | 38 | 8.3 | 38 | 8.3 | 0.228 | 48.7 | LOS D | 1.1 | 8.0 | 0.93 | 0.74 | 0.93 | 7.3 |
| 8 | T1 | 65 | 3.2 | 65 | 3.2 | 0.561 | 49.9 | LOS D | 2.7 | 19.4 | 1.00 | 0.78 | 1.02 | 20.6 |
| 9 | R2 | 23 | 4.5 | 23 | 4.5 | 0.561 | 54.5 | LOS D | 2.7 | 19.4 | 1.00 | 0.78 | 1.02 | 14.5 |
| Approach | | 126 | 5.0 | 126 | 5.0 | 0.561 | 50.4 | LOS D | 2.7 | 19.4 | 0.98 | 0.77 | 0.99 | 16.5 |
| West: Parkes Street | | | | | | | | | | | | | | |
| 10 | L2 | 99 | 1.1 | 99 | 1.1 | 0.756 | 32.2 | LOS C | 13.5 | 98.4 | 0.90 | 0.84 | 0.93 | 22.2 |
| 11 | T1 | 726 | 5.4 | 726 | 5.4 | 0.756 | 28.3 | LOS B | 13.5 | 98.4 | 0.91 | 0.85 | 0.95 | 16.8 |
| 12 | R2 | 97 | 3.3 | 97 | 3.3 | 0.756 | 36.3 | LOS C | 11.0 | 80.0 | 0.92 | 0.87 | 0.98 | 28.7 |
| Approach | | 922 | 4.7 | 922 | 4.7 | 0.756 | 29.6 | LOS C | 13.5 | 98.4 | 0.91 | 0.85 | 0.95 | 19.4 |
| All Vehicles | | 2198 | 4.2 | 2171 ^{N1} | 4.2 | 0.781 | 30.1 | LOS C | 13.5 | 98.4 | 0.87 | 0.78 | 0.90 | 22.4 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

^{N1} Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|--------------------------------------|------------|--------------|---------------------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian ped | Distance m | Prop. Queued | Effective Stop Rate | |
| P1 | South Full Crossing | 53 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | |
| P2 | East Full Crossing | 53 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | |
| P3 | North Full Crossing | 53 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | |
| P4 | West Full Crossing | 53 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | |
| All Pedestrians | | 211 | 44.3 | LOS E | | | 0.94 | 0.94 | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

MOVEMENT SUMMARY

 Site: 204 [Wigram St Parkes St EX + FU PM]

 Network: N202 [Parkes Street Network EX + FU PM]

Wigram Street and Harris Street
4:30-5:30

Site Category: Future PM

Signals - Fixed Time Coordinated Cycle Time = 100 seconds (Network Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------|------|--------------------|------|-----------|---------------|------------------|---------------------|------------|--------------|---------------------|------------------|---------------|
| Mov ID | Turn | Demand Flows | | Arrival Flows | | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue | | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed |
| | | Total veh/h | HV % | Total veh/h | HV % | | | | Vehicles | Distance m | | | | |
| South: Wigram Street | | | | | | | | | | | | | | |
| 1 | L2 | 85 | 6.2 | 85 | 6.2 | 0.210 | 32.8 | LOS C | 2.5 | 18.3 | 0.79 | 0.72 | 0.79 | 28.2 |
| 2 | T1 | 84 | 1.3 | 84 | 1.3 | 0.905 | 54.0 | LOS D | 3.3 | 23.4 | 0.93 | 0.95 | 1.39 | 19.4 |
| 3 | R2 | 33 | 0.0 | 33 | 0.0 | 0.905 | 70.5 | LOS F | 3.3 | 23.4 | 1.00 | 1.06 | 1.67 | 14.8 |
| Approach | | 202 | 3.1 | 202 | 3.1 | 0.905 | 47.7 | LOS D | 3.3 | 23.4 | 0.88 | 0.87 | 1.18 | 21.7 |
| East: Parkes Street | | | | | | | | | | | | | | |
| 4 | L2 | 40 | 5.3 | 39 | 5.4 | 0.827 | 29.8 | LOS C | 11.8 | 84.9 | 0.87 | 0.81 | 0.94 | 30.9 |
| 5 | T1 | 905 | 2.6 | 873 | 2.6 | 0.827 | 31.8 | LOS C | 13.3 | 95.2 | 0.93 | 0.87 | 1.00 | 22.8 |
| Approach | | 945 | 2.7 | 912 ^{N1} | 2.7 | 0.827 | 31.7 | LOS C | 13.3 | 95.2 | 0.93 | 0.87 | 1.00 | 23.2 |
| North: Wigram Street | | | | | | | | | | | | | | |
| 7 | L2 | 49 | 0.0 | 49 | 0.0 | 0.229 | 39.4 | LOS C | 1.4 | 10.0 | 0.85 | 0.74 | 0.85 | 8.8 |
| 8 | T1 | 145 | 1.4 | 145 | 1.4 | 1.147 | 185.2 | LOS F | 13.1 | 94.6 | 0.99 | 1.63 | 2.58 | 7.8 |
| 9 | R2 | 54 | 7.8 | 54 | 7.8 | 1.147 | 197.6 | LOS F | 13.1 | 94.6 | 1.00 | 1.68 | 2.67 | 4.7 |
| Approach | | 248 | 2.5 | 248 | 2.5 | 1.147 | 158.9 | LOS F | 13.1 | 94.6 | 0.97 | 1.46 | 2.26 | 7.1 |
| West: Parkes Street | | | | | | | | | | | | | | |
| 10 | L2 | 80 | 1.3 | 80 | 1.3 | 1.168 | 218.3 | LOS F | 42.3 | 300.7 | 1.00 | 2.04 | 2.66 | 4.4 |
| 11 | T1 | 837 | 1.9 | 837 | 1.9 | 1.168 | 213.7 | LOS F | 42.3 | 300.7 | 1.00 | 2.01 | 2.68 | 3.0 |
| 12 | R2 | 92 | 5.7 | 92 | 5.7 | 1.168 | 220.5 | LOS F | 35.9 | 257.0 | 1.00 | 1.98 | 2.69 | 7.9 |
| Approach | | 1008 | 2.2 | 1008 | 2.2 | 1.168 | 214.6 | LOS F | 42.3 | 300.7 | 1.00 | 2.01 | 2.68 | 3.6 |
| All Vehicles | | 2404 | 2.5 | 2371 ^{N1} | 2.5 | 1.168 | 124.2 | LOS F | 42.3 | 300.7 | 0.96 | 1.42 | 1.86 | 7.6 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

^{N1} Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|--------------------------------------|------------|--------------|---------------------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian ped | Distance m | Prop. Queued | Effective Stop Rate | |
| P1 | South Full Crossing | 53 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | |
| P2 | East Full Crossing | 53 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | |
| P3 | North Full Crossing | 53 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | |
| P4 | West Full Crossing | 53 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | |
| All Pedestrians | | 211 | 44.3 | LOS E | | | 0.94 | 0.94 | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.



Appendix C-3

Existing + Development with Improvements

MOVEMENT SUMMARY

 Site: 301 [Harris St Parkes St EX + FU AM Improvements]

 Network: N301 [Parkes Street Network EX + FU AM Improvements]

Harris Street and Parkes Street
7.45-8.45

Site Category: Improved Future AM

Signals - Fixed Time Coordinated Cycle Time = 80 seconds (Network Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------|------|---------------|------|-----------|---------------|------------------|---------------------|------------|--------------|---------------------|------------------|---------------|
| Mov ID | Turn | Demand Flows | | Arrival Flows | | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue | | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed |
| | | Total veh/h | HV % | Total veh/h | HV % | | | | Vehicles | Distance m | | | | |
| South: Harris Street | | | | | | | | | | | | | | |
| 1 | L2 | 49 | 10.6 | 49 | 10.6 | 1.069 | 126.4 | LOS F | 13.0 | 93.1 | 1.00 | 1.63 | 2.44 | 4.9 |
| 2 | T1 | 440 | 0.7 | 440 | 0.7 | 1.069 | 123.2 | LOS F | 13.0 | 93.1 | 1.00 | 1.62 | 2.45 | 11.1 |
| 3 | R2 | 34 | 3.1 | 34 | 3.1 | 1.069 | 126.8 | LOS F | 12.5 | 88.2 | 1.00 | 1.62 | 2.45 | 11.1 |
| Approach | | 523 | 1.8 | 523 | 1.8 | 1.069 | 123.7 | LOS F | 13.0 | 93.1 | 1.00 | 1.62 | 2.45 | 10.6 |
| East: Parkes Street | | | | | | | | | | | | | | |
| 4 | L2 | 44 | 4.8 | 44 | 4.8 | 0.669 | 24.3 | LOS B | 10.6 | 77.6 | 0.84 | 0.76 | 0.84 | 34.4 |
| 5 | T1 | 520 | 5.9 | 520 | 5.9 | 1.040 | 19.8 | LOS B | 14.9 | 107.1 | 0.85 | 0.76 | 0.86 | 29.8 |
| 6 | R2 | 325 | 3.2 | 325 | 3.2 | 1.040 | 108.3 | LOS F | 14.9 | 107.1 | 1.00 | 1.35 | 2.20 | 14.8 |
| Approach | | 889 | 4.9 | 889 | 4.9 | 1.040 | 52.4 | LOS D | 14.9 | 107.1 | 0.90 | 0.98 | 1.35 | 19.4 |
| North: Harris Street | | | | | | | | | | | | | | |
| 7 | L2 | 95 | 3.3 | 95 | 3.3 | 0.503 | 23.0 | LOS B | 6.8 | 47.9 | 0.80 | 0.72 | 0.80 | 32.7 |
| 8 | T1 | 279 | 0.8 | 279 | 0.8 | 0.503 | 19.5 | LOS B | 6.8 | 47.9 | 0.80 | 0.72 | 0.80 | 29.7 |
| 9 | R2 | 373 | 2.0 | 373 | 2.0 | 0.864 | 44.7 | LOS D | 8.8 | 62.6 | 1.00 | 1.21 | 1.78 | 16.4 |
| Approach | | 746 | 1.7 | 746 | 1.7 | 0.864 | 32.6 | LOS C | 8.8 | 62.6 | 0.90 | 0.96 | 1.29 | 22.9 |
| West: Parkes Street | | | | | | | | | | | | | | |
| 10 | L2 | 299 | 2.8 | 299 | 2.8 | 1.000 | 53.0 | LOS D | 12.6 | 90.9 | 1.00 | 1.17 | 1.69 | 19.3 |
| 11 | T1 | 501 | 6.7 | 501 | 6.7 | 1.000 | 42.8 | LOS D | 12.6 | 90.9 | 1.00 | 1.13 | 1.52 | 23.1 |
| Approach | | 800 | 5.3 | 800 | 5.3 | 1.000 | 46.6 | LOS D | 12.6 | 90.9 | 1.00 | 1.15 | 1.58 | 21.4 |
| All Vehicles | | 2959 | 3.6 | 2959 | 3.6 | 1.069 | 58.4 | LOS E | 14.9 | 107.1 | 0.95 | 1.13 | 1.59 | 17.8 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|--------------------------------------|------------|--------------|---------------------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian ped | Distance m | Prop. Queued | Effective Stop Rate | |
| P1 | South Full Crossing | 53 | 34.3 | LOS D | 0.1 | 0.1 | 0.93 | 0.93 | |
| P2 | East Full Crossing | 53 | 34.3 | LOS D | 0.1 | 0.1 | 0.93 | 0.93 | |
| P3 | North Full Crossing | 53 | 34.3 | LOS D | 0.1 | 0.1 | 0.93 | 0.93 | |
| P4 | West Full Crossing | 53 | 34.3 | LOS D | 0.1 | 0.1 | 0.93 | 0.93 | |
| All Pedestrians | | 211 | 34.3 | LOS D | | | 0.93 | 0.93 | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

MOVEMENT SUMMARY

 Site: 303 [Harris St Parkes St EX + FU PM Improvements]

 Network: N302 [Parkes Street Network EX + FU PM Improvements]

Harris Street and Parkes Street

4:30-5:30

Site Category: Improved Future PM

Signals - Fixed Time Coordinated Cycle Time = 72 seconds (Network Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------|------|---------------|------|-----------|---------------|------------------|---------------------|------------|--------------|---------------------|------------------|---------------|
| Mov ID | Turn | Demand Flows | | Arrival Flows | | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue | | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed |
| | | Total veh/h | HV % | Total veh/h | HV % | | | | Vehicles | Distance m | | | | |
| South: Harris Street | | | | | | | | | | | | | | |
| 1 | L2 | 53 | 0.0 | 53 | 0.0 | 1.012 | 84.4 | LOS F | 8.2 | 57.5 | 1.00 | 1.42 | 2.20 | 7.2 |
| 2 | T1 | 360 | 0.6 | 360 | 0.6 | 1.012 | 80.0 | LOS F | 8.2 | 57.5 | 1.00 | 1.42 | 2.20 | 16.4 |
| 3 | R2 | 31 | 0.0 | 31 | 0.0 | 1.012 | 84.8 | LOS F | 8.0 | 56.0 | 1.00 | 1.42 | 2.21 | 15.5 |
| Approach | | 443 | 0.5 | 443 | 0.5 | 1.012 | 80.9 | LOS F | 8.2 | 57.5 | 1.00 | 1.42 | 2.20 | 15.5 |
| East: Parkes Street | | | | | | | | | | | | | | |
| 4 | L2 | 32 | 0.0 | 32 | 0.0 | 0.667 | 23.4 | LOS B | 9.0 | 64.2 | 0.86 | 0.76 | 0.86 | 35.2 |
| 5 | T1 | 494 | 2.6 | 494 | 2.6 | 1.148 | 21.0 | LOS B | 16.0 | 112.5 | 0.86 | 0.78 | 0.90 | 30.4 |
| 6 | R2 | 258 | 0.4 | 258 | 0.4 | 1.148 | 185.4 | LOS F | 16.0 | 112.5 | 1.00 | 1.76 | 3.20 | 10.2 |
| Approach | | 783 | 1.7 | 783 | 1.7 | 1.148 | 75.2 | LOS F | 16.0 | 112.5 | 0.91 | 1.10 | 1.65 | 15.2 |
| North: Harris Street | | | | | | | | | | | | | | |
| 7 | L2 | 78 | 2.7 | 78 | 2.7 | 0.604 | 24.1 | LOS B | 7.9 | 55.5 | 0.84 | 0.78 | 0.84 | 38.2 |
| 8 | T1 | 379 | 0.8 | 379 | 0.8 | 0.604 | 19.6 | LOS B | 7.9 | 55.5 | 0.84 | 0.78 | 0.84 | 34.9 |
| 9 | R2 | 436 | 1.4 | 436 | 1.4 | 1.019 | 69.0 | LOS E | 14.9 | 105.9 | 1.00 | 1.20 | 2.09 | 11.0 |
| Approach | | 893 | 1.3 | 893 | 1.3 | 1.019 | 44.1 | LOS D | 14.9 | 105.9 | 0.92 | 0.98 | 1.45 | 19.3 |
| West: Parkes Street | | | | | | | | | | | | | | |
| 10 | L2 | 320 | 0.7 | 320 | 0.7 | 1.018 | 57.8 | LOS E | 12.9 | 91.4 | 1.00 | 1.19 | 1.73 | 17.7 |
| 11 | T1 | 593 | 2.3 | 593 | 2.3 | 1.018 | 44.1 | LOS D | 13.7 | 98.1 | 1.00 | 1.20 | 1.64 | 21.7 |
| Approach | | 913 | 1.7 | 913 | 1.7 | 1.018 | 48.9 | LOS D | 13.7 | 98.1 | 1.00 | 1.20 | 1.67 | 20.1 |
| All Vehicles | | 3032 | 1.4 | 3032 | 1.4 | 1.148 | 59.0 | LOS E | 16.0 | 112.5 | 0.95 | 1.14 | 1.68 | 17.6 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|--------------------------------------|------------|--------------|---------------------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian ped | Distance m | Prop. Queued | Effective Stop Rate | |
| P1 | South Full Crossing | 53 | 30.3 | LOS D | 0.1 | 0.1 | 0.92 | 0.92 | |
| P2 | East Full Crossing | 53 | 30.3 | LOS D | 0.1 | 0.1 | 0.92 | 0.92 | |
| P3 | North Full Crossing | 53 | 30.3 | LOS D | 0.1 | 0.1 | 0.92 | 0.92 | |
| P4 | West Full Crossing | 53 | 30.3 | LOS D | 0.1 | 0.1 | 0.92 | 0.92 | |
| All Pedestrians | | 211 | 30.3 | LOS D | | | 0.92 | 0.92 | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

MOVEMENT SUMMARY

 Site: 302 [Wigram St Parkes St EX + FU AM Improvements]

 Network: N301 [Parkes Street Network EX + FU AM Improvements]

Wigram Street and Harris Street

7.45-8.45

Site Category: Improved Future AM

Signals - Fixed Time Coordinated Cycle Time = 80 seconds (Network Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------|------|--------------------|------|-----------|---------------|------------------|---------------------|------------|--------------|---------------------|------------------|---------------|
| Mov ID | Turn | Demand Flows | | Arrival Flows | | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue | | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed |
| | | Total veh/h | HV % | Total veh/h | HV % | | | | Vehicles | Distance m | | | | |
| South: Wigram Street | | | | | | | | | | | | | | |
| 1 | L2 | 54 | 11.8 | 54 | 11.8 | 0.277 | 41.6 | LOS C | 1.2 | 9.6 | 0.95 | 0.74 | 0.95 | 24.6 |
| 2 | T1 | 139 | 0.8 | 139 | 0.8 | 0.754 | 41.5 | LOS C | 4.1 | 29.1 | 1.00 | 0.91 | 1.21 | 22.9 |
| 3 | R2 | 22 | 0.0 | 22 | 0.0 | 0.754 | 46.0 | LOS D | 4.1 | 29.1 | 1.00 | 0.91 | 1.21 | 20.2 |
| Approach | | 215 | 3.4 | 215 | 3.4 | 0.754 | 42.0 | LOS C | 4.1 | 29.1 | 0.99 | 0.87 | 1.14 | 23.1 |
| East: Parkes Street | | | | | | | | | | | | | | |
| 4 | L2 | 35 | 3.0 | 35 | 3.0 | 0.730 | 31.8 | LOS C | 10.3 | 74.7 | 0.95 | 0.85 | 0.98 | 30.0 |
| 5 | T1 | 900 | 3.7 | 897 | 3.7 | 0.730 | 31.4 | LOS C | 11.2 | 80.7 | 0.98 | 0.87 | 1.01 | 23.0 |
| Approach | | 935 | 3.7 | 931 ^{N1} | 3.7 | 0.730 | 31.4 | LOS C | 11.2 | 80.7 | 0.98 | 0.87 | 1.01 | 23.3 |
| North: Wigram Street | | | | | | | | | | | | | | |
| 7 | L2 | 38 | 8.3 | 38 | 8.3 | 0.191 | 41.0 | LOS C | 0.9 | 6.5 | 0.94 | 0.72 | 0.94 | 8.4 |
| 8 | T1 | 65 | 3.2 | 65 | 3.2 | 0.416 | 37.5 | LOS C | 2.1 | 15.0 | 0.97 | 0.76 | 0.97 | 24.0 |
| 9 | R2 | 23 | 4.5 | 23 | 4.5 | 0.416 | 42.1 | LOS C | 2.1 | 15.0 | 0.97 | 0.76 | 0.97 | 17.5 |
| Approach | | 126 | 5.0 | 126 | 5.0 | 0.416 | 39.4 | LOS C | 2.1 | 15.0 | 0.96 | 0.75 | 0.96 | 19.3 |
| West: Parkes Street | | | | | | | | | | | | | | |
| 10 | L2 | 99 | 1.1 | 99 | 1.1 | 0.687 | 19.6 | LOS B | 12.3 | 89.5 | 0.78 | 0.72 | 0.78 | 30.6 |
| 11 | T1 | 726 | 5.4 | 726 | 5.4 | 0.687 | 17.5 | LOS B | 12.3 | 89.5 | 0.81 | 0.75 | 0.83 | 23.0 |
| 12 | R2 | 97 | 3.3 | 97 | 3.3 | 0.687 | 41.4 | LOS C | 5.1 | 36.9 | 0.99 | 0.86 | 1.07 | 26.3 |
| Approach | | 922 | 4.7 | 922 | 4.7 | 0.687 | 20.2 | LOS B | 12.3 | 89.5 | 0.83 | 0.76 | 0.85 | 24.5 |
| All Vehicles | | 2198 | 4.2 | 2195 ^{N1} | 4.2 | 0.754 | 28.2 | LOS B | 12.3 | 89.5 | 0.91 | 0.82 | 0.95 | 23.3 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

^{N1} Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|--------------------------------------|------------|--------------|---------------------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian ped | Distance m | Prop. Queued | Effective Stop Rate | |
| P1 | South Full Crossing | 53 | 34.3 | LOS D | 0.1 | 0.1 | 0.93 | 0.93 | |
| P2 | East Full Crossing | 53 | 34.3 | LOS D | 0.1 | 0.1 | 0.93 | 0.93 | |
| P3 | North Full Crossing | 53 | 34.3 | LOS D | 0.1 | 0.1 | 0.93 | 0.93 | |
| P4 | West Full Crossing | 53 | 34.3 | LOS D | 0.1 | 0.1 | 0.93 | 0.93 | |
| All Pedestrians | | 211 | 34.3 | LOS D | | | 0.93 | 0.93 | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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MOVEMENT SUMMARY

 Site: 304 [Wigram St Parkes St EX + FU PM Improvements]

 Network: N302 [Parkes Street Network EX + FU PM Improvements]

Wigram Street and Harris Street

4:30-5:30

Site Category: Improved Future PM

Signals - Fixed Time Coordinated Cycle Time = 72 seconds (Network Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------|------|--------------------|------|-----------|---------------|------------------|---------------------|------------|--------------|---------------------|------------------|---------------|
| Mov ID | Turn | Demand Flows | | Arrival Flows | | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue | | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed |
| | | Total veh/h | HV % | Total veh/h | HV % | | | | Vehicles | Distance m | | | | |
| South: Wigram Street | | | | | | | | | | | | | | |
| 1 | L2 | 85 | 6.2 | 85 | 6.2 | 0.429 | 38.9 | LOS C | 1.8 | 13.5 | 0.97 | 0.76 | 0.97 | 25.6 |
| 2 | T1 | 84 | 1.3 | 84 | 1.3 | 0.592 | 35.4 | LOS C | 2.6 | 18.2 | 1.00 | 0.81 | 1.05 | 24.6 |
| 3 | R2 | 33 | 0.0 | 33 | 0.0 | 0.592 | 40.0 | LOS C | 2.6 | 18.2 | 1.00 | 0.81 | 1.05 | 21.9 |
| Approach | | 202 | 3.1 | 202 | 3.1 | 0.592 | 37.6 | LOS C | 2.6 | 18.2 | 0.99 | 0.79 | 1.02 | 24.7 |
| East: Parkes Street | | | | | | | | | | | | | | |
| 4 | L2 | 40 | 5.3 | 40 | 5.3 | 0.892 | 41.5 | LOS C | 11.6 | 83.3 | 1.00 | 1.05 | 1.29 | 26.0 |
| 5 | T1 | 905 | 2.6 | 903 | 2.6 | 0.892 | 40.9 | LOS C | 12.1 | 86.2 | 1.00 | 1.05 | 1.29 | 19.4 |
| Approach | | 945 | 2.7 | 942 ^{N1} | 2.7 | 0.892 | 40.9 | LOS C | 12.1 | 86.2 | 1.00 | 1.05 | 1.29 | 19.7 |
| North: Wigram Street | | | | | | | | | | | | | | |
| 7 | L2 | 49 | 0.0 | 49 | 0.0 | 0.257 | 37.2 | LOS C | 1.0 | 7.2 | 0.94 | 0.74 | 0.94 | 9.1 |
| 8 | T1 | 145 | 1.4 | 145 | 1.4 | 0.841 | 40.4 | LOS C | 4.9 | 35.1 | 1.00 | 1.01 | 1.39 | 23.1 |
| 9 | R2 | 54 | 7.8 | 54 | 7.8 | 0.841 | 45.0 | LOS D | 4.9 | 35.1 | 1.00 | 1.01 | 1.39 | 16.6 |
| Approach | | 248 | 2.5 | 248 | 2.5 | 0.841 | 40.8 | LOS C | 4.9 | 35.1 | 0.99 | 0.96 | 1.30 | 19.9 |
| West: Parkes Street | | | | | | | | | | | | | | |
| 10 | L2 | 80 | 1.3 | 80 | 1.3 | 0.846 | 29.2 | LOS C | 16.2 | 115.3 | 0.93 | 0.97 | 1.09 | 24.0 |
| 11 | T1 | 837 | 1.9 | 837 | 1.9 | 0.846 | 26.9 | LOS B | 16.2 | 115.3 | 0.94 | 0.98 | 1.15 | 17.6 |
| 12 | R2 | 92 | 5.7 | 92 | 5.7 | 0.846 | 43.6 | LOS D | 7.0 | 50.0 | 1.00 | 1.02 | 1.35 | 25.9 |
| Approach | | 1008 | 2.2 | 1008 | 2.2 | 0.846 | 28.6 | LOS C | 16.2 | 115.3 | 0.95 | 0.98 | 1.16 | 19.5 |
| All Vehicles | | 2404 | 2.5 | 2401 ^{N1} | 2.5 | 0.892 | 35.4 | LOS C | 16.2 | 115.3 | 0.98 | 0.99 | 1.21 | 20.2 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

^{N1} Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|--------------------------------------|------------|--------------|---------------------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian ped | Distance m | Prop. Queued | Effective Stop Rate | |
| P1 | South Full Crossing | 53 | 30.3 | LOS D | 0.1 | 0.1 | 0.92 | 0.92 | |
| P2 | East Full Crossing | 53 | 30.3 | LOS D | 0.1 | 0.1 | 0.92 | 0.92 | |
| P3 | North Full Crossing | 53 | 30.3 | LOS D | 0.1 | 0.1 | 0.92 | 0.92 | |
| P4 | West Full Crossing | 53 | 30.3 | LOS D | 0.1 | 0.1 | 0.92 | 0.92 | |
| All Pedestrians | | 211 | 30.3 | LOS D | | | 0.92 | 0.92 | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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